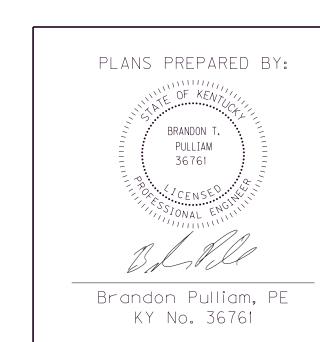
## TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

# GRAYSON COUNTY KY-224 over WESTERN KENTUCKY PARKWAY STA. 50+00.00

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	BID ITEM CODE	08100	08104	08150	08151	08002	08003	08020	02231	08046	08033	02998	08670	03299	25028ED	23378EC	02403	08305	08094	25078ED	08130	08133	08269
	BID ITEM	CONCRETE-CLASS A	CONCRETE-CLASS AA	STEEL REINFORCEMENT	STEEL REINFORCEMENT- EPOXY COATED	STRUCTURE EXCAV-SOLID ROCK	FOUNDATION PREPARATION	CRUSHED AGGREGATE SLOPE PROT	STRUCTURE GRANULAR BACKFILL	PILES-STEEL HP12X53	TEST PILES	MASONRY COATING	PRECAST PC BOX BEAM SB27	ARMORED EDGE FOR CONCRETE	RAIL SYSTEM SINGLE SLOPE 40 IN	CONCRETE SEALING	REMOVE CONCRETE MASONRY	REMOVE REINF CONCRETE	PILE POINTS-12 IN	THRIE BEAM GUARDRAIL TRANSITION TL-3	MECHANICAL REINF COUPLER #5	MECHANICAL REINF COUPLER #8	ELECTRICAL CONDUIT
	UNIT	CUYD	CUYD	LB	LB	CUYD	LS	TON	CUYD	LF	LF	SQYD	LF	LF	LF	SQFT	CUYD	LS	EACH	EACH	EACH	EACH	LS
	END BENT #1	40.1		2774				121	167	154	27	40					24.4		8	3			
ייי	PIER #1	128.7		19852		15						239					28.3				15	14	
TURE	PIER #2	154.4		19864	120	15						390					29.3				15	14	
U	PIER #3	128.5		19558		15						390					27.3				15	14	
SUBSTRU	END BENT #2	47.8		3017				145	164	81	17	49					25.6		8	3			
UB																							
°																							
	SUPERSTRUCTURE		387.9		100884								1401.0	113.6	479	19012	426.9			4			1
B	RIDGE TOTALS	499.5	387.9	65065	101004	45	1	266	331	235	44	1108	1401.0	113.6	479	19012	561.8	1	16	4	45	42	1



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## **GENERAL NOTES**

#### **SPECIFICATIONS**

ALL REFERENCES TO THE STANDARD SPECIFICATIONS ARE TO THE 2019 EDITION OF THE KENTUCKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION WITH CURRENT SUPPLEMENTAL SPECIFICATIONS. ALL REFERENCES TO THE AASHTO SPECIFICATIONS ARE TO THE 9TH EDITION OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS WITH INTERIMS.

#### **DESIGN LOAD**

THIS BRIDGE IS DESIGNED FOR A MODIFIED HL-93 (KY HL-93) LIVE LOAD OBTAINED BY INCREASING THE AASHTO LOADING BY 25%. SEE EXISTING PLANS FOR THE DESIGN LOAD USED FOR THE SUBSTRUCTURE THAT WILL REMAIN IN-PLACE.

#### **FUTURE WEARING SURFACE**

THIS BRIDGE IS DESIGNED FOR A 15 PSF FUTURE WEARING SURFACE LOAD.

#### **DESIGN METHOD**

ALL REINFORCED CONCRETE MEMBERS ARE DESIGNED BY THE LOAD AND RESISTANCE FACTOR METHOD AS SPECIFIED IN THE CURRENT AASHTO SPECIFICATIONS. SEE EXISTING PLANS FOR THE DESIGN METHOD USED FOR THE SUBSTRUCTURE THAT WILL REMAIN IN-PLACE.

#### MATERIALS DESIGN SPECIFICATIONS

F'C = 3500 PSI FOR CLASS "A" REINFORCED CONCRETE F'C = 4000 PSI FOR CLASS "AA" REINFORCED CONCRETE Fy = 60000 PSI FOR STEEL REINFORCEMENT

FOR PRESTRESSED BEAM MATERIAL SPECIFICATIONS, SEE BEAM SHEET.

#### MATERIAL SPECIFICATIONS

ASTM OR AASHTO SPECIFICATIONS, CURRENT EDITION, AS DESIGNATED BELOW SHALL GOVERN THE MATERIALS FURNISHED.

AASHTO M-270, GRADE 36

AASHTO M-203, GRADE 270

ASTM A615/A615M-96a

STRUCTURAL STEEL, 36,000 PSI MIN. YIELD
UNCOATED SEVEN-WIRE LOW-RELAXATION STRAND
FOR PRESTRESSED CONCRETE
STEEL REINFORCEMENT, GRADE 60

#### **CONCRETE**

CLASS "AA" CONCRETE IS TO BE USED IN THE SUPERSTRUCTURE. CLASS "A" CONCRETE IS TO BE USED IN THE SUBSTRUCTURE. PRESTRESSED BEAM CONCRETE SHALL BE IN ACCORDANCE WITH THE PLANS AND SPECIFICATONS.

#### STRUCTURE GRANULAR BACKFILL

EXCAVATION INTO EXISTING PAVEMENT OR GROUND BEHIND THE ABUTMENTS THAT MAY BE REQUIRED FOR ABUTMENT CONSTRUCTION SHALL BE BACKFILLED WITH STRUCTURE GRANULAR BACKFILL IN ACCORDANCE WITH SPECIAL PROVISION 69. WRAP ALL ROCK IN GEOTEXTILE FABRIC CLASS 2. ALL GEOTEXTILE FABRIC SHALL BE INCIDENTAL TO THE UNIT PRICE BID FOR STRUCTURE GRANULAR BACKFILL.

#### **MASONRY COATING**

CONTRARY TO THE SPECIFICATIONS, ONLY APPLY MASONRY COATING TO THE SUBSTRUCTURES. THE ENTIRE EXPOSED SUBSTRUCTURE INCLUDING THE EXISTING PORTIONS OF THE ABUTMENTS SHALL RECIEVE MASONRY COATING TO 6" BELOW THE FINISHED GROUND LINE.

#### **ON-SITE INSPECTION**

EACH CONTRACTOR SUBMITTING A BID FOR THIS WORK SHALL MAKE A THOROUGH INSPECTION OF THE PROJECT SITE PRIOR TO SUBMITTING A BID AND SHALL BE THOROUGHLY FAMILIARIZED WITH EXISTING CONDITIONS SO THAT WORK CAN BE EXPEDITIOUSLY PERFORMED AFTER A CONTRACT IS AWARDED. SUBMISSION OF A BID WILL BE CONSIDERED EVIDENCE OF THIS INSPECTION HAVING BEEN MADE. ANY CLAIMS RESULTING FROM SITE CONDITIONS WILL NOT BE HONORED BY THE DEPARTMENT OF HIGHWAYS.

#### DAMAGE TO THE SUBSTRUCTURES

THE CONTRACTOR IS RESPONSIBLE FOR ANY AND ALL DAMAGES TO THE EXISTING SUBSTRUCTURES DURING RECONSTRUCTION EVEN TO THE REPLACEMENT OF THE ENTIRE SUBSTRUCTURE, SHOULD THEY BE DAMAGED DUE TO HIS ACTIONS.

#### **PILING**

PILING SHALL BE DRIVEN TO PRACTICAL REFUSAL. TEST PILES SHALL BE DRIVEN WHERE DESIGNATED ON THE PLANS TO DETERMINE THE LENGTH OF PILE REQUIRED. ALL TEST PILES SHALL BE ACCURATELY LOCATED SO THAT THEY MAY BE USED IN THE FINISHED STRUCTURE.

#### PILE POINTS

PILE POINTS ARE REQUIRED. THE POINTS SHALL BE THE TYPE FOR KEYING INTO A SLOPING ROCK SURFACE. SEE SECTION 604.03.04(C) OF THE STANDARD SPECIFICATIONS.

#### **SLOPE PROTECTION**

SLOPE PROTECTION SHALL BE CRUSHED AGGREGATE SLOPE PROTECTION IN ACCORDANCE WITH SECTION 805.13 OF THE SPECIFICATIONS. GEOTEXTILE FABRIC UNDER THE SLOPE PROTECTION SHALL BE CONSIDERED INCIDENTAL TO THE UNIT PRICE BID FOR CRUSHED AGGREGATE SLOPE PROTECTION.

#### REINFORCEMENT

DIMENSIONS SHOWN FROM THE FACE OF CONCRETE TO BARS ARE TO CENTER OF BARS UNLESS OTHERWISE SHOWN OR NOTED. SPACING OF BARS IS FROM CENTER TO CENTER OF BARS. THE CLEAR DISTANCE TO FACE OF CONCRETE IS 2" UNLESS OTHERWISE NOTED. ANY REINFORCING BARS DESIGNATED BY THE SUFFIX (E) IN THE PLANS SHALL BE EPOXY COATED IN ACCORDANCE WITH SECTON 811.10 OF THE STANDARD SPECIFICATIONS. ANY REINFORCING BARS DESIGNATED BY THE SUFFIX (S) IN THE BENDING DIAGRAM SHALL BE CONSIDERED A STIRRUP FOR PURPOSES OF BEND DIAMETERS.

#### **CONTRACTOR VERIFY DIMENSIONS**

CONTRACTOR SHALL VERIFY DIMENSION AND ELEVATIONS SHOWN IN THE PLANS BEFORE AND DURING CONSTRUCTION AND ADJUST BAR LENGTHS AND OR BEAM LENGTHS TO ENSURE PROPER FIT AND FINISH IN THE FINAL PRODUCT. DIMENSIONS ARE FOR A NORMAL TEMPERATURE OF 60 DEGREES FAHRENHEIT. LAYOUT DIMENSIONS ARE HORIZONTAL DIMENSIONS.

#### **BEVELED EDGES**

ALL EXPOSED EDGES SHALL BE BEVELED  $\frac{3}{4}$ " UNLESS OTHERWISE NOTED.

#### COMPLETION OF THE STRUCTURE

THE CONTRACTOR IS REQUIRED TO COMPLETE THE STRUCTURE IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. MATERIAL, LABOR OR CONSTRUCTION OPERATIONS, NOT OTHERWISE SPECIFIED, ARE TO BE INCLUDED IN THE BID ITEM MOST APPROPRIATE TO THE WORK INVOLVED. THIS MAY INCLUDE COFFERDAMS, SHORING, EXCAVATIONS, BACKFILLING, REMOVAL OF ALL OR PARTS OF EXISTING STRUCTURES, PHASE CONSTRUCTION, INCIDENTAL MATERIALS, LABOR, OR ANYTHING ELSE REQUIRED TO COMPLETE THE STRUCTURE.

#### **BEFORE YOU DIG**

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL REQUIREMENTS AND CONFORMATION WITH THE UNDERGROUND FACILITY DAMAGE PREVENTION ACT OF 1994. THE CONTRACTOR WILL BE RESPONSIBLE FOR LOCATING ANY UTILITIES ON THIS PROJECT. ALL UNDERGROUND UTILITIES SHALL BE LOCATED PRIOR TO CONSTRUCTION. THE CONTRACTOR IS ADVISED TO CALL (800) 752-6007 A MINIMUM OF TWO WORKING DAYS PRIOR TO EXCAVATION FOR INFORMATION ON THE LOCATION OF SOME BUT NOT NECESSARILY ALL UNDERGROUND UTILITIES.

#### STAY-IN-PLACE METAL FORMS

THE USE OF STAY-IN-PLACE FORMWORK FOR THE BRIDGE DECK IS PERMITTED PROVIDED THE CORRUGATIONS ARE FILLED WITH EXPANDED POLYSTYRENE.

## **EXISTING REINFORCING STEEL**

THE COST OF CUTTING, BENDING AND CLEANING EXISTING REINFORCING STEEL IS TO BE INCIDENTAL TO THE LUMP SUM BID FOR REMOVE CONCRETE MASONRY.

#### FOUNDATION PREPARATION

INCLUDE IN THIS LUMP SUM BID THE COST OF ANY REQUIRED COMMON EXCAVATION (INCLUDING MATERIALS, LABOR, EQUIPMENT, ETC.) IN ACCORDANCE WITH SECTION 603 OF THE SPECIFICATIONS. CONTRARY TO THE SPECIFICATIONS, BACKFILLING BEHIND THE ABUTMENTS SHALL BE PAID FOR IN THE UNIT PRICE BID FOR STRUCTURE GRANULAR BACKFILL.

#### **CONCRETE SEALER**

SUPERSTRUCTURE AREAS DETAILED IN THE SPECIFICATIONS AS REQUIRING MASONRY COATING SHALL BE SEALED IN ACCORDANCE WITH THE SPECIAL NOTE FOR CONCRETE SEALING AND IN THE AREAS SHOWN IN THE DETAIL ON S21. CONCRETE SURFACES (EXCEPT THE DECK) SHALL RECEIVE THE ORDINARY SURFACE FINISH AS DESCRIBED IN SECTION 601.03.18(A) PRIOR TO BEING SEALED.

#### **EXISTING HANDRAIL**

REMOVE AND RELOCATE THE EXISTING ALUMINUM HANDRAIL AS DIRECTED BY THE ENGINEER. ALL COSTS TO REMOVE, DELIVER TO A LOCATION AS SPECIFIED BY THE ENGINEER, OR DISPOSAL FEES SHALL BE INCIDENTAL TO THE BID FOR REMOVE CONCRETE MASONRY.

#### **ELECTRICAL CONDUIT**

THE LUMP SUM BID FOR THIS ITEM SHALL INCLUDE FURNISHING ALL CONDUIT, JUNCTION BOXES, ANCHOR BOLTS, OTHER RELATED MATERIALS, AND ALL LABOR NECESSARY FOR PLACEMENT IN ACCORDANCE WITH THE PLANS, SPECIFICATIONS, AND STANDARD DRAWINGS. ALL CONDUIT SHALL BE RIGID GALVANIZED CONDUIT. CONDUIT SHALL EXTEND 4' BEYOND END OF BRIDGE AND BE CAPPED. MARK END LOCATION WITH CONCRETE MARKER, 4 REQUIRED. LOCATION OF JUNCTION BOXES SHALL BE AS DIRECTED BY THE ENGINEER.

#### REMOVE REINFORCED CONCRETE

THE LUMP SUM BID FOR THIS ITEM SHALL INCLUDE ALL LABOR, MATERIALS, AND DISPOSAL COSTS ASSOCIATED WITH THE REMOVAL OF THE EXISTING PAVED CONCRETE SLOPEWALLS AT EACH ABUTMENT. THE ESTIMATED QUANTITY FOR THIS REMOVAL IS 88 CY AT ABUTMENT 1 AND 90 CY AT ABUTMENT 2.

#### **TEMPORARY SUPPORTS**

TEMPORARY SUPPORTS OR SHORING WILL NOT BE PERMITTED UNDER THE BEAMS WHEN POURING THE CONCRETE DECK SLAB OR WHEN TAKING "TOP OF BEAM" ELEVATIONS.

#### STRUCTURE EXCAVATION

SHEETING OR SHORING MAY BE NECESSARY FOR CONSTRUCTION. THE COST OF ANY SUCH WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR FOUNDATION PREPARATION OR REMOVE CONCRETE MASONRY, AS APPROPRIATE.

#### **CONSTRUCTION IDENTIFICATION**

THE NAMES OF THE PRIME CONTRACTOR AND THE SUBCONTRACTOR SHALL BE IMPRINTED IN THE CONCRETE WITH ONE INCH LETTERS AT A LOCATION DESIGNATED BY THE ENGINEER. THE CONTRACTOR SHALL FURNISH ALL PLANS, EQUIPMENT AND LABOR NECESSARY TO DO THE WORK FOR WHICH NO DIRECT PAYMENT WILL BE MADE. SEE STD. DWG. BGX-006, C.E.

#### **DISCLAIMER**

ACCEPTANCE OF ANY CONTRACTOR'S SUBMISSION REQUIRED ON THIS PROJECT DOES NOT CONSTITUTE ENDORSEMENT OR APPROVAL. THE ACCEPTANCE IS ACKNOWLEDGEMENT OF THE WORK PERFORMED AND AUTHORIZATION FOR THE CONTRACTOR TO PROCEED. THE DEPARTMENT IS NOT BOUND BY ACCEPTANCE OF ANY SUBMISSIONS REQUIRED. FINAL ACCEPTANCE OR APPROVAL WILL BE CONTINGENT ON THE SATISFACTORY COMPLETION OF THE PROJECT.

#### SHOP DRAWINGS

WHEN CHANGES IN THE SHOP DRAWINGS ARE PROPOSED BY THE FABRICATOR OR SUPPLIER, THE SHOP DRAWINGS REFLECTING THESE CHANGES SHALL BE SUBMITTED TO THE CONSULTANT THROUGH THE CONTRACTOR. THE CONSULTANT SHALL PROVIDE THE DIVISION OF STRUCTURAL DESIGN ONE COPY OF THE FINAL APPROVED SHOP PLANS.

#### SLAB POURING SEQUENCE

SLAB SHALL BE POURED CONTINUOUSLY OUT TO OUT.

#### **EXISTING PLANS**

EXISTING PLANS CAN BE FOUND UNDER DRAWING NUMBER 14991.

#### **BONDED CONSTRUCTION JOINT**

WHERE A BONDED CONSTRUCTION JOINT IS CALLED FOR IN THE PLANS, BOND NEW PLASTIC CONCRETE TO HARDENED CONCRETE USING A TYPE V EPOXY RESIN OR OTHER APPROVED STRUCTURAL ADHESIVE AS PRESCRIBED IN SECTION 826 OF THE SPECIFICATIONS. FOLLOW THE MANUFACTURER'S RECOMMENDED APPLICATION INSTRUCTIONS. THIS WORK AND MATERIAL IS INCIDENTAL TO THE UNIT PRICE BIDS FOR CLASS "A" OR CLASS "AA" CONCRETE.

#### DRILLING AND GROUTING

IN ACCORDANCE WITH SECTION 826 OF THE SPECIFICATIONS, DRILL HOLES TO A DEPTH AS SHOWN HEREIN THESE PLANS AND APPLY A TYPE IV EPOXY BONDING ADHESIVE IN THE HOLES. ALSO, APPLY A TYPE V EPOXY BONDING MATERIAL TO THE INTERFACE BETWEEN THE EXISTING CONCRETE AND THE NEW CONCRETE PRIOR TO PLACING THE NEW CONCRETE. ALL COSTS ASSOCIATED WITH THIS WORK SHALL BE INCIDENTAL TO THE UNIT PRICE BID FOR CLASS "A" CONCRETE.

#### REMOVE CONCRETE MASONRY

THE CONTRACTOR SHALL USE HAND HELD JACK HAMMERS OR HYDRO-DEMOLITION
TECHNIQUES TO REMOVE CONCRETE WITHOUT DAMAGING THE EXISTING REINFORCEMENT
THAT IS TO REMAIN IN PLACE. ANY CONCRETE REMOVAL OUTSIDE THE DETAILED LIMITS
SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE. THE CONTRACTOR SHALL MAKE A
SAW CUT AT THE REMOVAL LIMITS TO FORM A NEAT CONSTRUCTION JOINT. ALL COSTS OF
THIS PROCEEDURE ARE INCLUDED IN THE PRICE BID FOR, "REMOVE CONCRETE MASONRY."

#### **MECHANICAL COUPLERS**

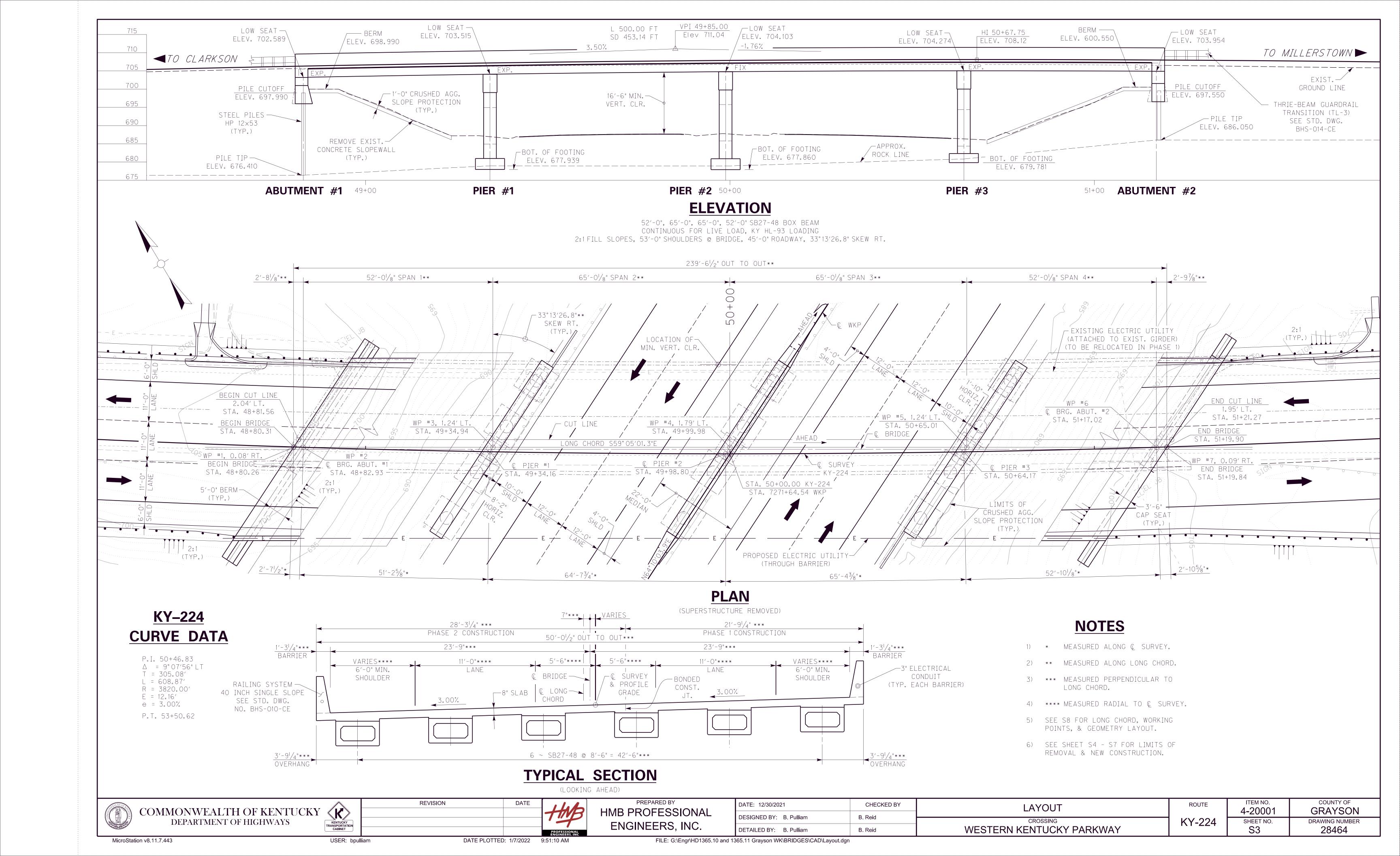
MECHANICAL COUPLERS SHALL BE IN ACCORDANCE WITH SECTION 602.03.06 OF THE SPECIFICATIONS. CONTRACTOR SHALL FURNISH TWO ADDITIONAL TEST SPECIMENS TO THE DIVISION OF MATERIALS FOR APPROVAL.

#### BONDING NEW CONCRETE TO OLD CONCRETE

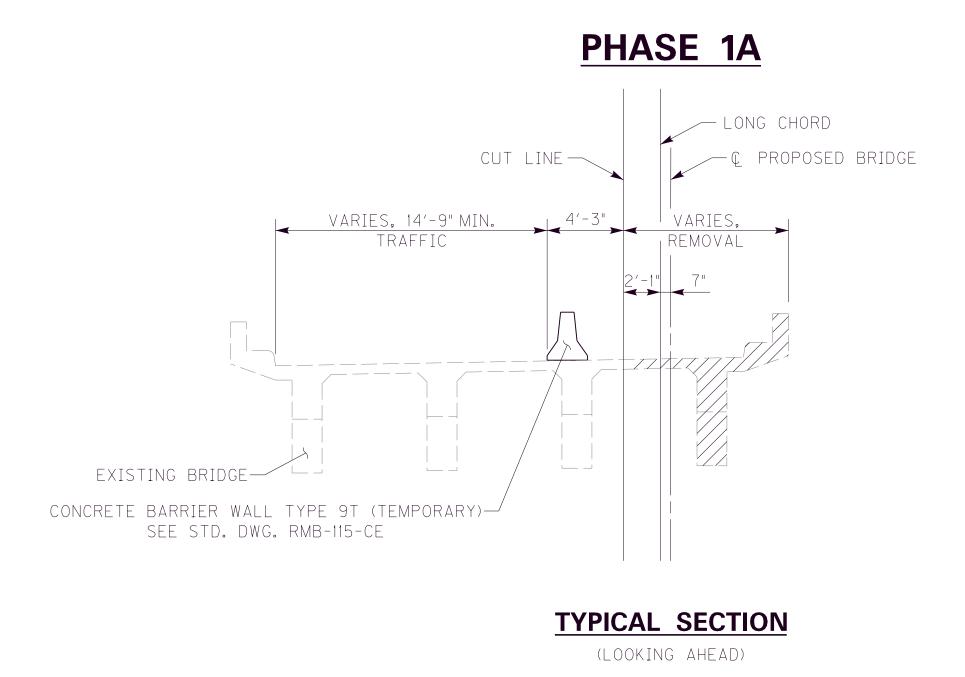
ALL NEW CONCRETE SHALL BE BONDED TO THE OLD CONCRETE WITH A TYPE V EPOXY RESIN SYSTEM CONFORMING TO SECTION 511 AND 826 OF THE SPECIFICATIONS. THE COST OF THIS WORK, INCLUDING ALL LABOR, TOOLS, AND MATERIALS IS TO BE INCIDENTAL TO THE UNIT PRICE BIDS FOR CLASS "A" OR CLASS "AA" CONCRETE.

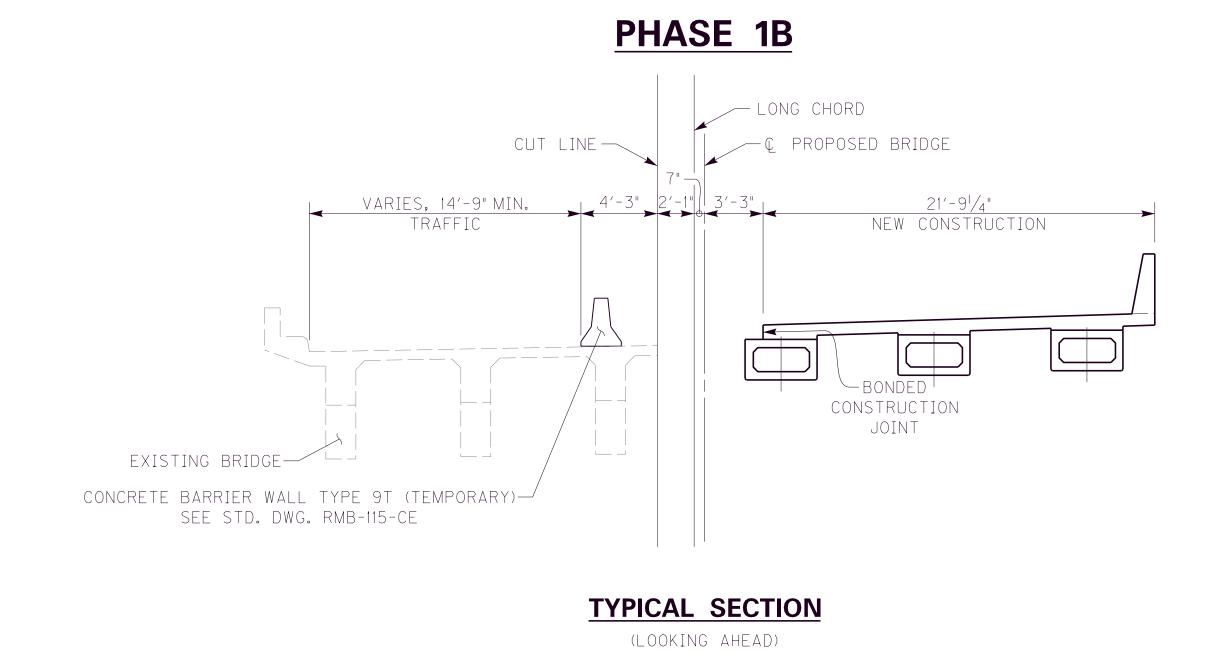
#### **SPIRAL COLUMN TIES**

SPLICES FOR SPIRALS WHERE DESIRED BY THE CONTRACTOR SHALL BE MADE WITH A MINIMUM OF ONE AND ONE-HALF TURNS OF SPIRAL. NO ADDITIONAL PAYMENT WILL BE MADE FOR THESE SPLICES, AND THE COST WILL BE CONSIDERED INCIDENTAL TO THE COST OF THE DEVELOPED-LENGTH OF SPIRAL SHOWN ON THE PLANS. SPIRAL REINFORCEMENT SHALL MEET THE REQUIREMENTS OF SECTION 811 OF THE SPECIFICATIONS.

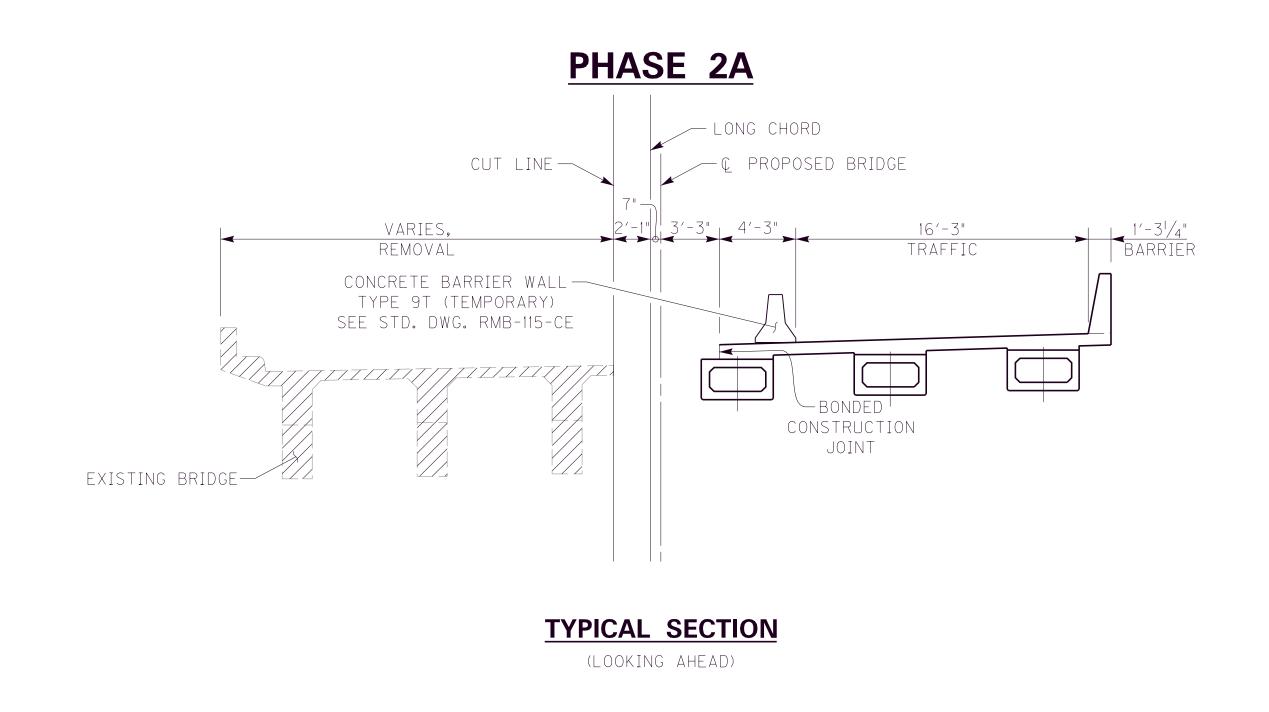


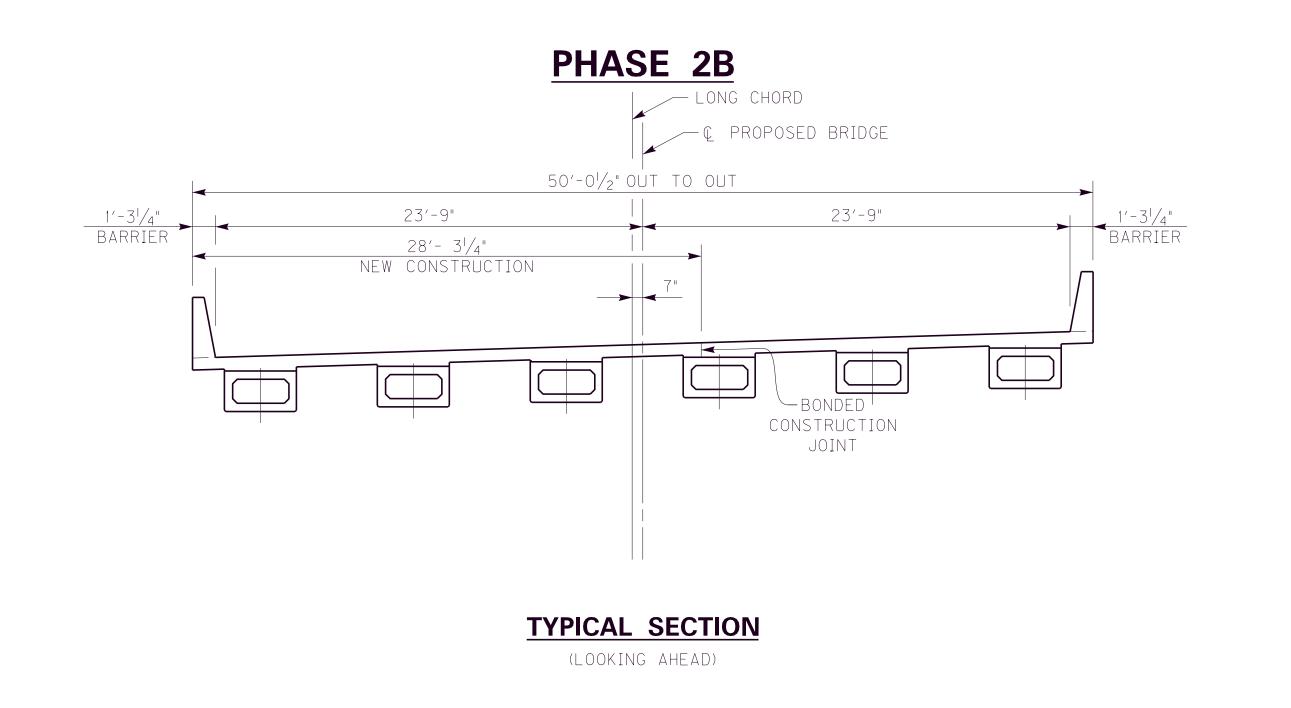
## PHASE 1 CONSTRUCTION

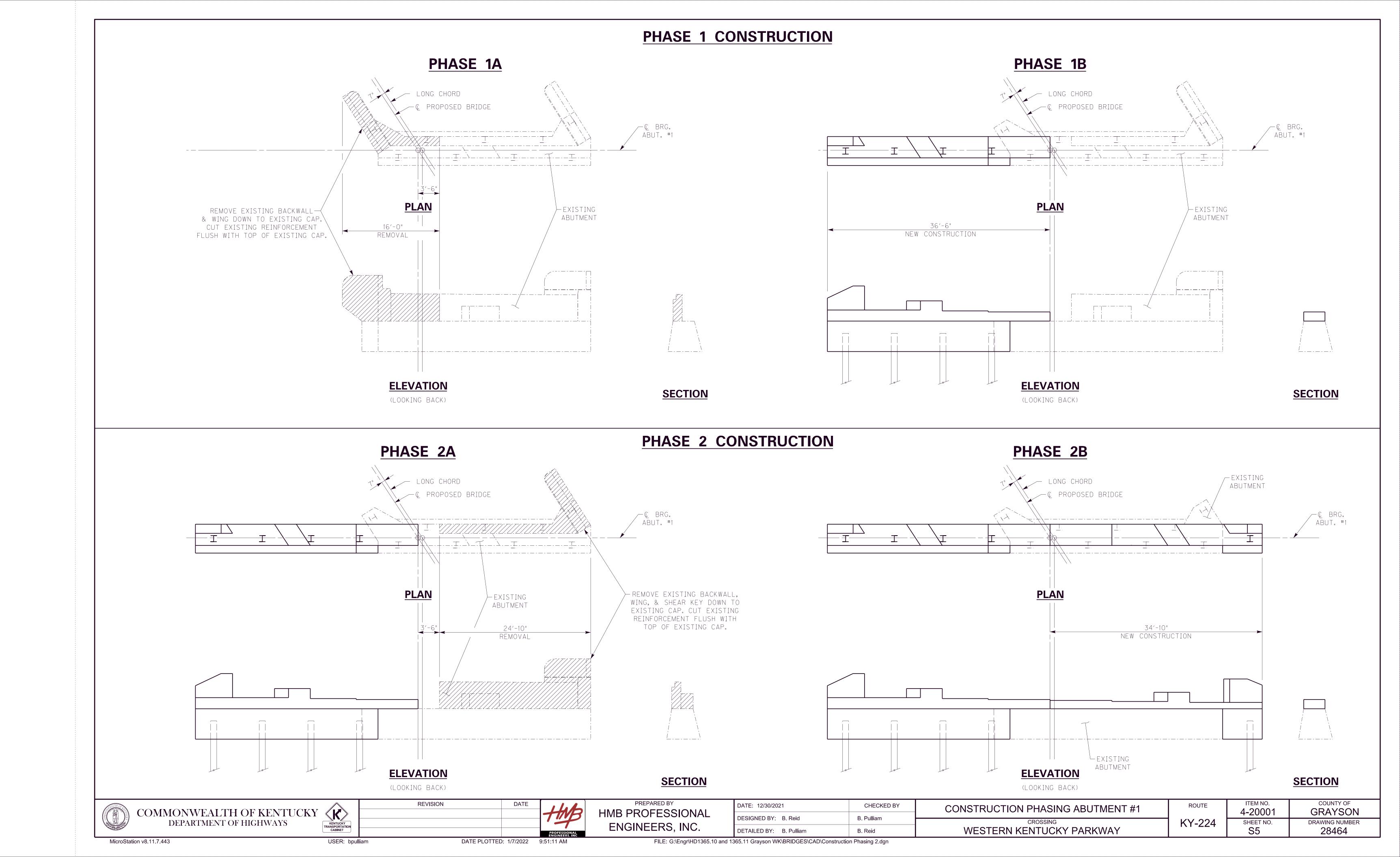


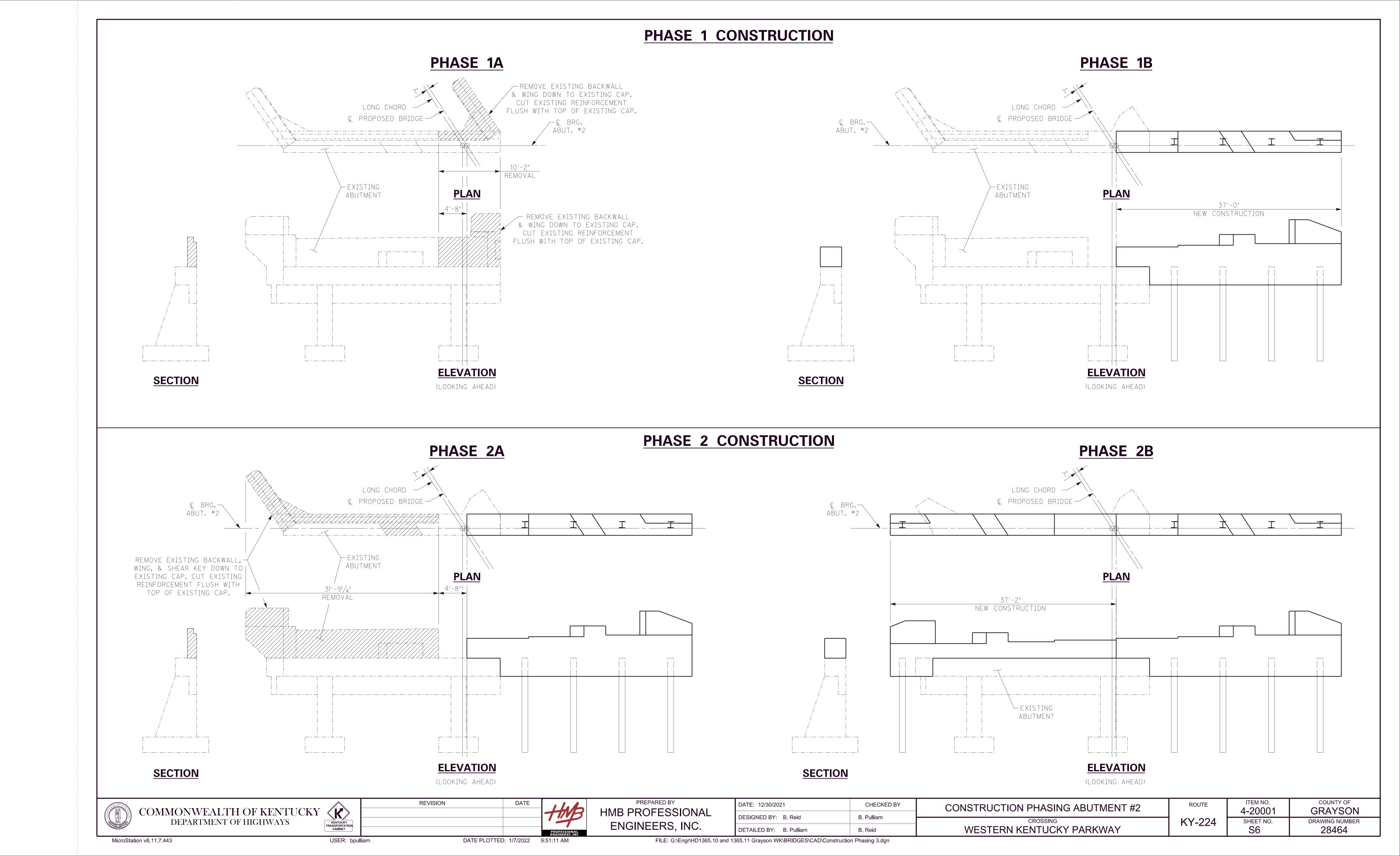


## PHASE 2 CONSTRUCTION



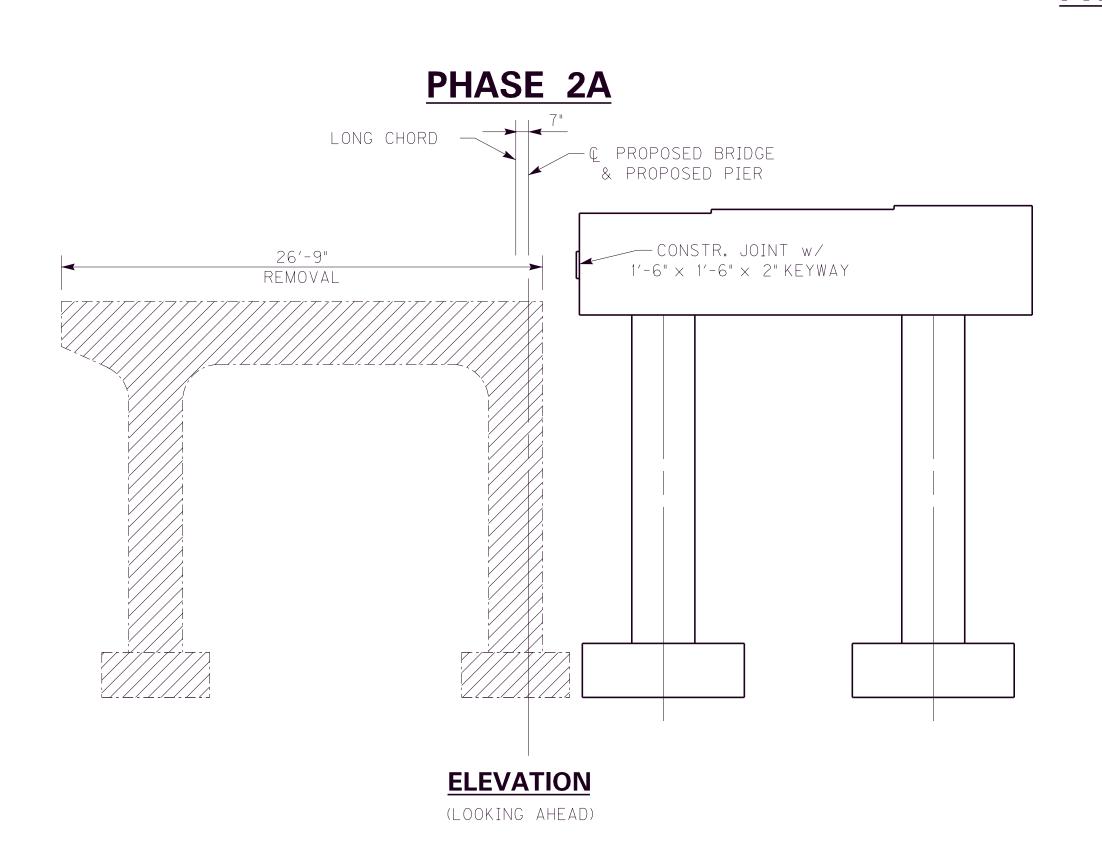






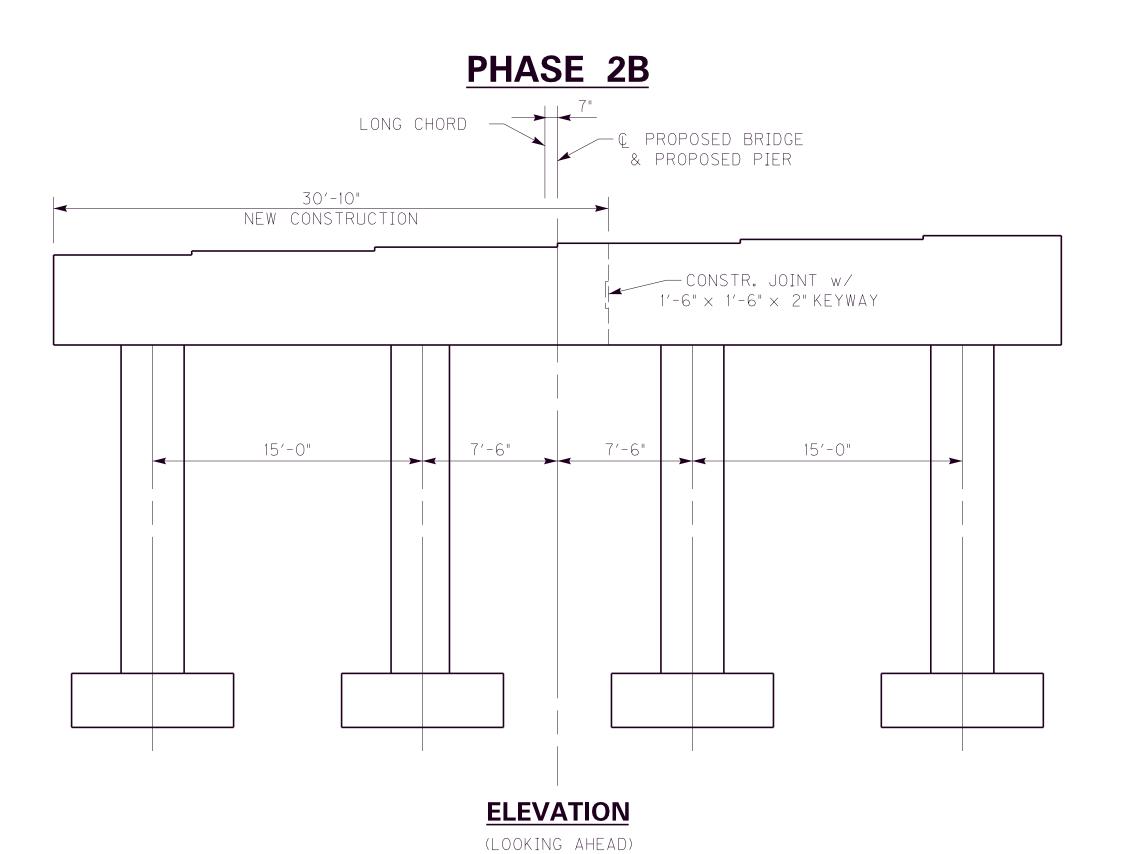


## PHASE 2 CONSTRUCTION



**ELEVATION** 

(LOOKING AHEAD)



**ELEVATION** 

(LOOKING AHEAD)

PREPARED BY HMB PROFESSIONAL ENGINEERS, INC.

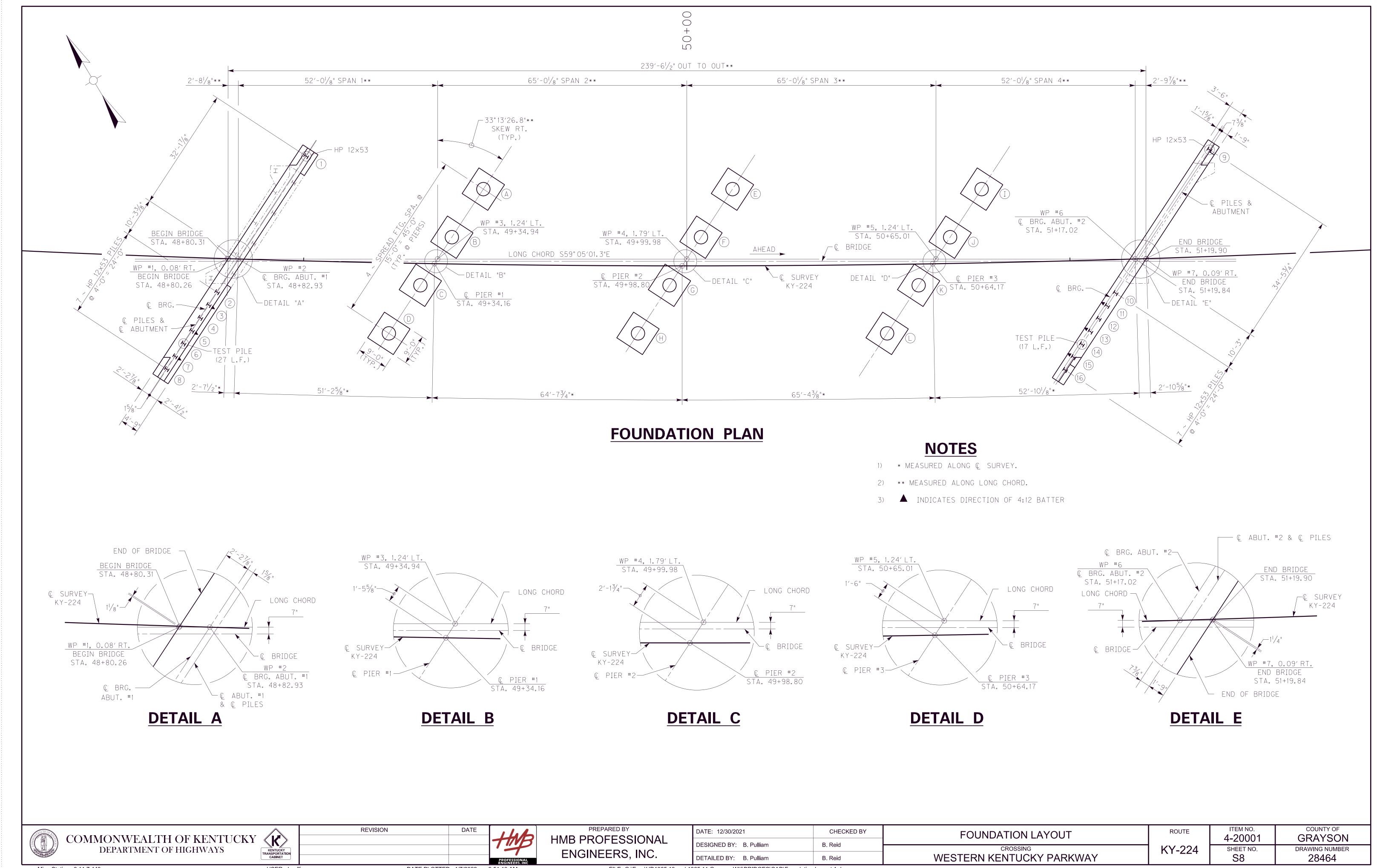
DATE: 12/30/2021 CHECKED BY DESIGNED BY: B. Reid B. Pulliam DETAILED BY: B. Pulliam

ROUTE CONSTRUCTION PHASING PIERS 4-20001 CROSSING KY-224 SHEET NO. WESTERN KENTUCKY PARKWAY

8" CLEARANCE PIER #1

81/2" CLEARANCE PIER #2

93/4" CLEARANCE PIER #3



## PILE RECORD FOR POINT BEARING PILES

PILE NO.	PILE CUT-OF ELEVATI (FEET	F ION	TIP OF PILE ELEVATION AS DRIVEN (FEET)	LENGTH OF PILE IN PLACE (FEET)	AXIAL	SIGN LOAD NS)			
1	697.99	90			72				
2									
3									
4									
5									
6									
7	<b>Y</b>								
8	697.99	90							
9	697.55	50							
10									
11									
12									
13									
14									
15	<b>Y</b>					1			
16	697.55	50			7	2			

## FIELD DATA

FOR EACH PILE, THE PROJECT ENGINEER SHALL RECORD THE FOLLOWING ON THIS SHEET: PILE LENGTH IN PLACE AND POINT OF PILE ELEVATION AS DRIVEN. SUBMIT THIS RECORD TO:

> DIRECTOR, DIVISION OF STRUCTURAL DESIGN ROOM #322 200 MERO STREET FRANKFORT, KY. 40622-0001

THIS PILE RECORD DOES NOT REPLACE OTHER PILE RECORDS THE PROJECT ENGINEER IS REQUIRED TO KEEP AND SUBMIT.

AFTER PIER FOUNDATIONS HAVE BEEN PLACED, THE PROJECT RESIDENT ENGINEER SHALL RECORD THE BOTTOM OF FOOTING ELEVATION "AS-BUILT" AND SHALL SUBMIT ONE COPY OF THIS SHEET WITH THIS DATA TO THE DIRECTOR, DIVISION OF BRIDGES.

USE HP 12X53 IN ACCORDANCE WITH BPS-003, C.E. USE GRADE 50 STEEL H-PILES WITH PILE POINTS FOR END BEARING PILES.

## **DEFINITIONS OF TERMS**

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure. PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished structure.

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elevation in the finished structure.

DESIGN AXIAL LOAD: Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.

CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

DRIVING CRITERIA: Drive point bearing piles to practical refusal.

PRACTICAL REFUSAL: FOR THIS PROJECT MINIMUM BLOW REQUIREMENTS ARE REACHED AFTER TOTAL PENETRATION BECOMES  $\frac{1}{2}$ " OR LESS FOR 10 CONSECUTIVE BLOWS, PRACTICAL REFUSAL IS OBTAINED AFTER THE PILE IS STRUCK AN ADDITIONAL 10 BLOWS WITH TOTAL PENETRATION OF 1/2" OR LESS. IMMEDIATELY CEASE DRIVING OPERATIONS IF THE PILE VISIBLY YIELDS OR BECOMES DAMAGED DURING DRIVING. DRIVE ADDITIONAL PRODUCTION AND TEST PILES IF DIRECTED BY THE ENGINEER.

FOUNDATION LAYOUT

CROSSING

WESTERN KENTUCKY PARKWAY

		SP	READ	FOOT	ING RECO	)RD		
	PIER	#		PIER	#2		PIER	#3
POINT	BOTTOM FTG. ELEV.	AS-BUILT FTG. ELEVATION	POINT	BOTTOM FTG. ELEV.	AS-BUILT FTG. ELEVATION	POINT	BOTTOM FTG. ELEV.	AS-BUILT FTG. ELEVATION
А	677.939		E	677.860		I	679.781	
В	677.939		F	677.860		J	679.781	
С	677.939		G	677.860		K	679.781	
D	677.939		Н	677.860		L	679.781	

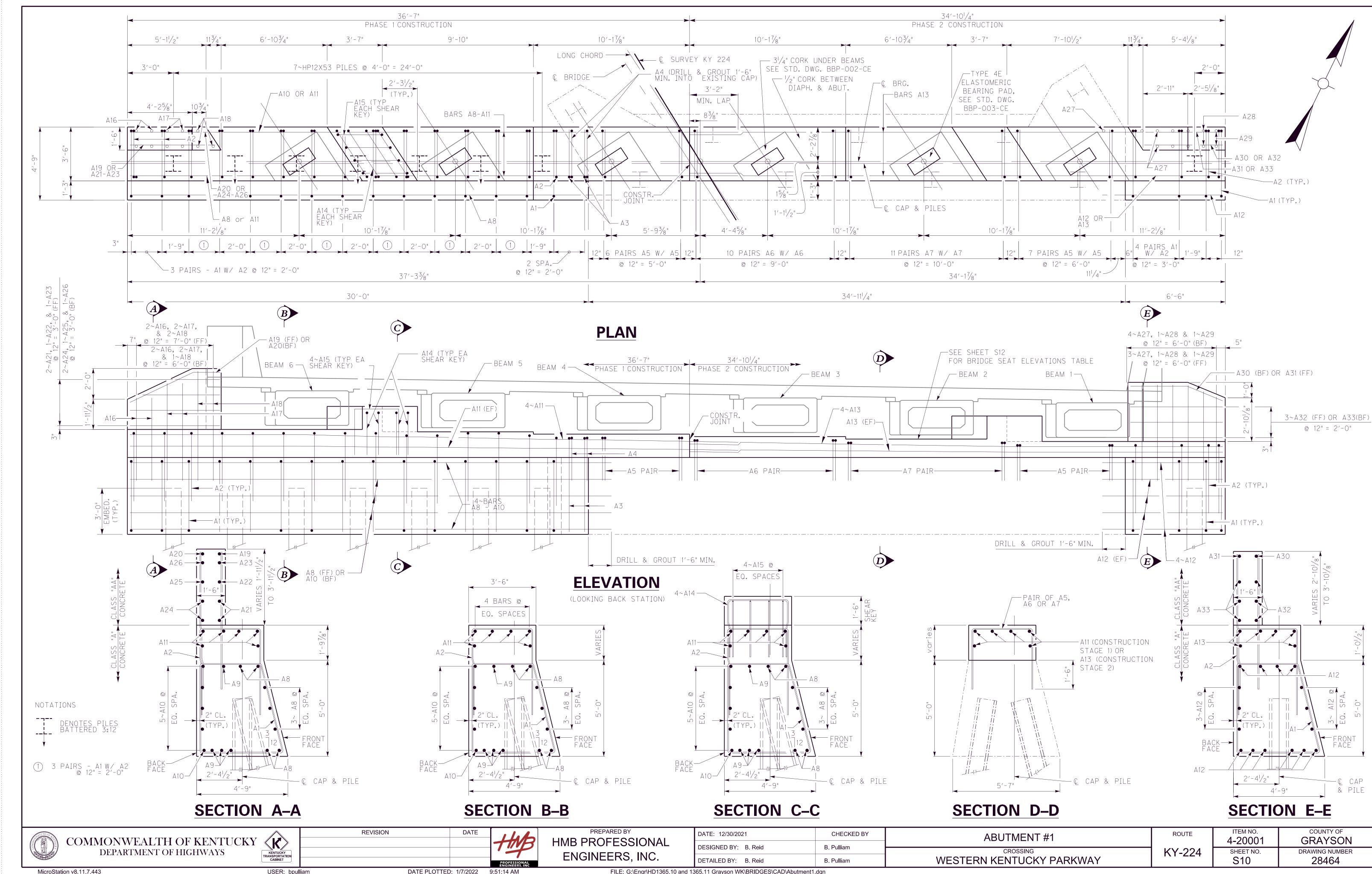
FOOTINGS ARE DESIGNED FOR A MAXIMUM SERVICE LIMIT BEARING PRESSURE OF 12 ksf.

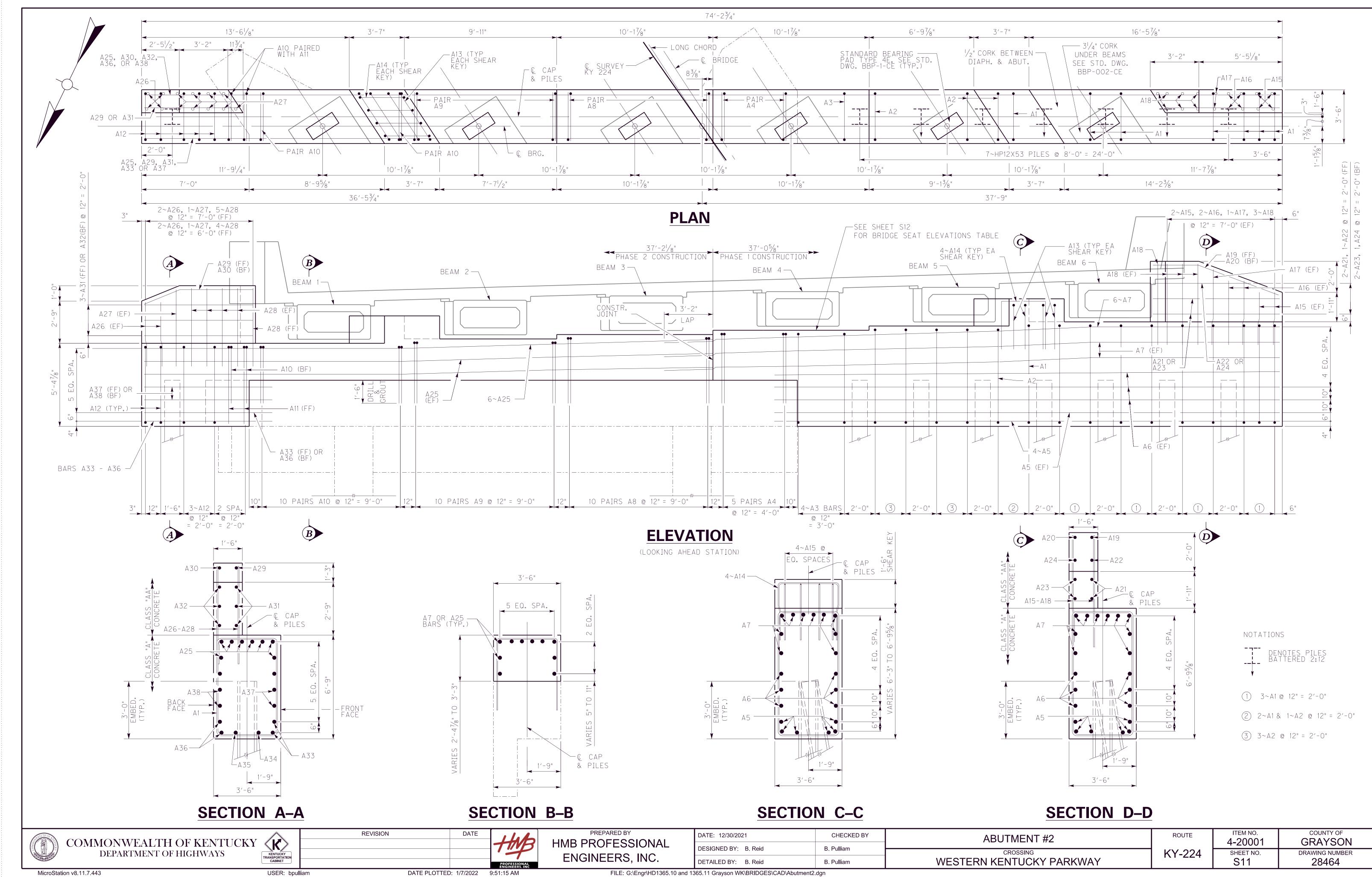
ENGINEERS, INC.

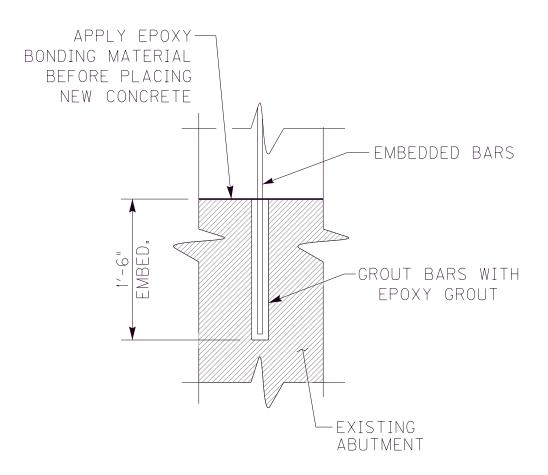
HMB PROFESSIONAL

DATE: 12/30/2021 CHECKED BY DESIGNED BY: B. Pulliam B. Reid DETAILED BY: B. Pulliam

COUNTY OF ROUTE 4-20001 GRAYSON KY-224 SHEET NO. DRAWING NUMBER 28464



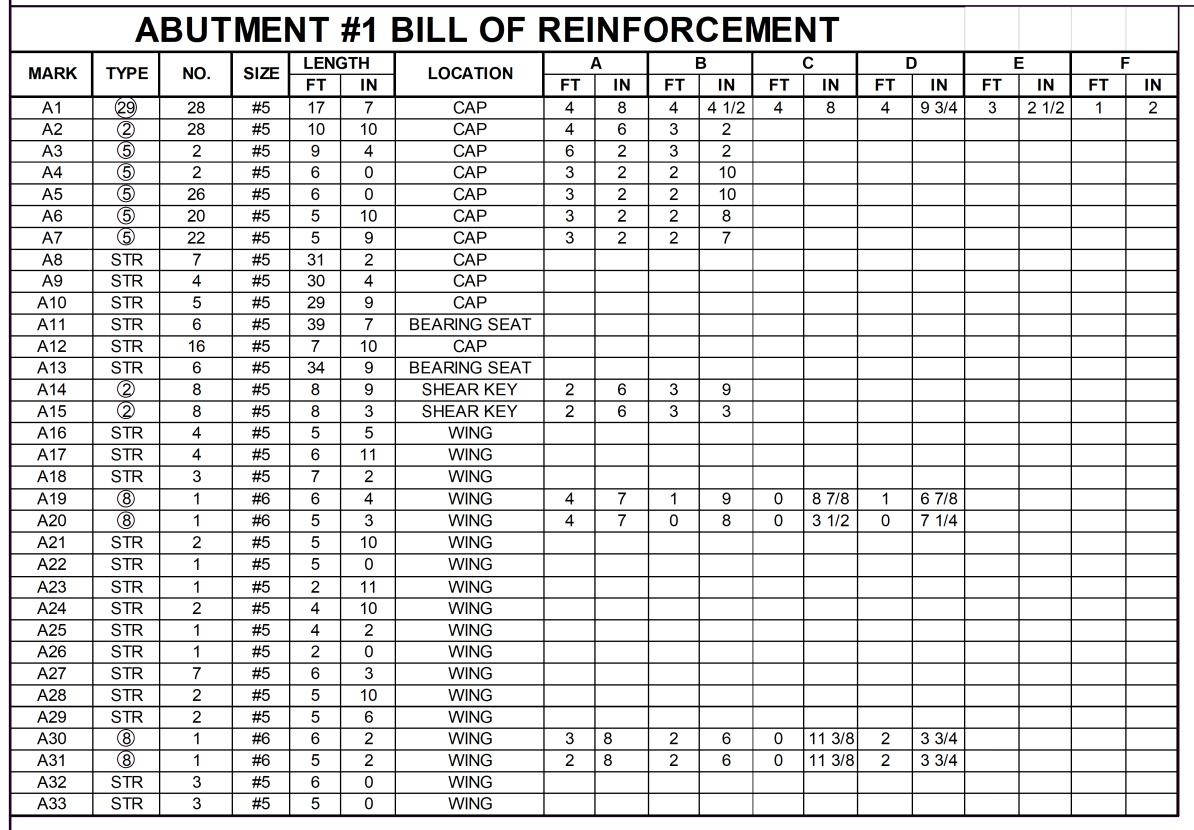


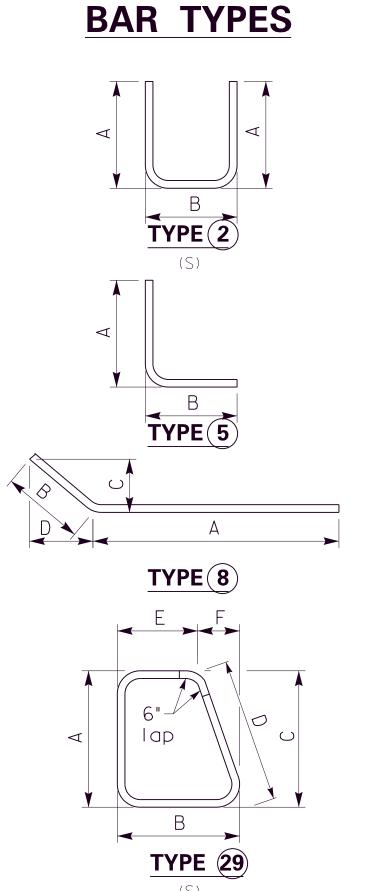


## **DRILL & GROUT DETAIL**

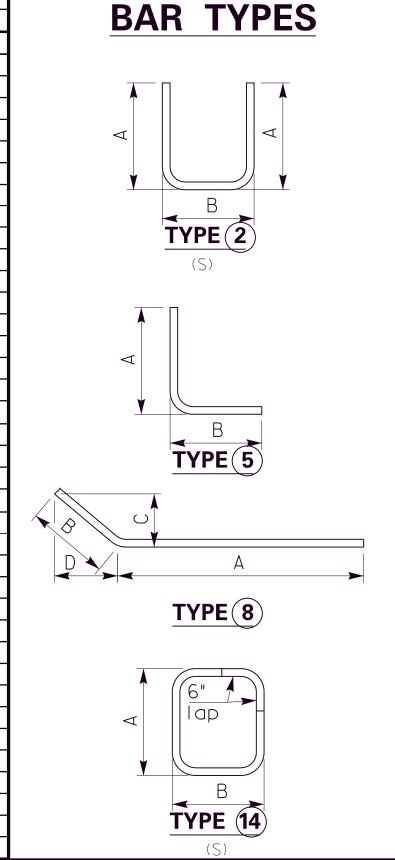
BEAM SEAT	<b>ELEVATIONS</b>
ABUTN	ΛΕΝΤ #1
BEAM 1	702.589
BEAM 2	702.750
BEAM 3	702.909
BEAM 4	703.066
BEAM 5	703.220
BEAM 6	703.373

<b>BEAM SEAT</b>	ELEVATIONS
ABUTM	1ENT #2
BEAM 1	703.954
BEAM 2	704.241
BEAM 3	704.524
BEAM 4	704.803
BEAM 5	705.080
BEAM 6	705.352





MARK         TYP           A1         (14)           A2         (14)           A3         (14)           A4         5           A5         STF           A6         STF           A8         (5)           A9         (5)           A10         (5)           A11         (5)           A12         (14)           A13         (2)           A14         (2)           A15         STF           A16         STF           A17         STF           A18         STF           A20         (8)           A21         STF           A22         STF           A23         STF           A24         STF	) 14 ) 7 ) 2 11 R 6 R 11 D 2 D 2 D 2 D 2 D 2 D 2 D 2 R 2 R 2 R 2 R 2 R 2 R 2 R 3 R 3 R 3 R 3 R 3 R 3 R 3 R 3 R 3 R 3	NO.       SIX         14       #         7       #         4       #         10       #         6       #         10       #         20       #         20       #         22       #         5       #         8       #         4       #         4       #         7       #         1       #	F1 5 19 5 18 5 7 6 32 5 40 5 7 5 6 6 8 5 17 5 9 5 4 5 9 5 4	8 2 8 9 11 11 1 6 2 11 3	CAP	FT 6 6 5 4 4 4 3 5 5 3 3 3	1N 5 2 11 7 4 0 9 1 1 2 2	3 3 3 3 3 3 3 3 3	2 2 2 2 2 2 2 2 2 2 9 3	FT		FT	IN	FT		FT	IN
A2 (14) A3 (14) A4 5 A5 STF A6 STF A7 STF A8 (5) A9 (5) A10 (5) A11 (5) A12 (14) A13 (2) A14 (2) A15 STF A16 STF A16 STF A18 STF A19 (8) A20 (8) A21 STF A22 STF A23 STF	) 7 ) 2 11 R 6 R 11 P 2 P 2 P 2 P 2 P 2 P 2 P 2 P 2 P 2 P 3 P 4 P 4 P 4 P 4 P 4 P 4 P 4 P 4 P 4 P 4	7 # 4 # 10 # 6 # 6 # 10 # 20 # 20 # 22 # 5 # 8 # 8 # 4 # 4 # 7 #	5 19 5 18 5 7 6 32 5 40 5 7 5 6 6 8 5 17 5 10 5 9 5 3 5 4 5 4 5 5 4	2 8 9 11 11 6 2 11 3 0 1 7 6 2 11	CAP	6 5 4 4 4 3 5 5 3	2 11 7 4 0 9 1 1 2	3 3 3 3 3 3 3	2 2 2 2 2 2 2 2 2 9								
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A4 5 A5 STF A6 STF A7 STF A8 5 A9 5 A10 5 A11 5 A12 14 A13 2 A14 2 A15 STF A16 STF A16 STF A18 STF A19 8 A20 8 A21 STF A22 STF A23 STF	1 1 2 6 R 6 R 7 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	10 # 6 # 6 # 10 # 20 # 20 # 22 # 5 # 8 # 8 # 4 # 4 # 7 #	5 7 6 32 5 40 5 7 6 7 6 6 6 8 6 17 6 9 6 3 6 4 6 4 6 4	9 11 11 6 2 11 3 0 1 7 6 2 11	CAP	4 4 4 3 5 5 3	7 4 0 9 1 1 2	3 3 3 3 3 3	2 2 2 2 2 2 9								
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A13 ② A14 ② A15 STF A16 STF A17 STF A18 STF A19 ⑧ A20 ⑧ A21 STF A22 STF A23 STF	1     8       2     2       3     4       4     2       7     7       1     1	8 # 8 # 4 # 4 # 2 # 7 #	5 10 5 9 5 3 5 4 5 4 5 5	1 7 6 2 11	SHEAR KEY SHEAR KEY WING WING WING	3	2	3	9								
A14 ② A15 STF A16 STF A17 STF A18 STF A19 8 A20 8 A21 STF A22 STF A23 STF	R       R       R       R       R       I       I	8 # 4 # 4 # 7 #	5 9 5 3 5 4 5 4 5 5	7 6 2 11	SHEAR KEY WING WING WING		<u> </u>										
A15 STF A16 STF A17 STF A18 STF A19 ® A20 ® A21 STF A22 STF A23 STF	R     4       R     4       R     2       R     7       1     1	4 # 4 # 2 # 7 #	5 3 5 4 5 4 5 5	6 2 11	WING WING WING	3	2	3	3								
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	7 2	2 #	5 8	3	WING										+		
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A25 STF		10 #			CAP										+		
A26 STF		4 #		2	WING										+		
A27 STF		2 #		0	WING										+		
A28 STF		9 #		1	WING						<del>                                     </del>				+		
A29 8		1 #		3	WING	4	9	2	6	0	11 3/8	2	4		†		
A30 8		1 #		3	WING	3	9	2	6	0	11 3/8	_ <del>_</del>	4		+		
A31 STF		3 #		1	WING	†		_					<del>                                     </del>		+		
A32 STF		3 #		1	WING						<del>                                     </del>				†		
A33 STF		2 #		4	CAP	1									+		
A34 STF		1 #		0	CAP	1					+ +				+		
A35 STF		1 #		1	CAP	+					<del>                                     </del>				+		
A36 STF		2 #		7	CAP	1					<del>                                     </del>				+		
A37 STF		2 #	<u> </u>	4	CAP	+					<del>                                     </del>				+		
A38 STF		2 #		7	CAP	+					+ +				+		



COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION

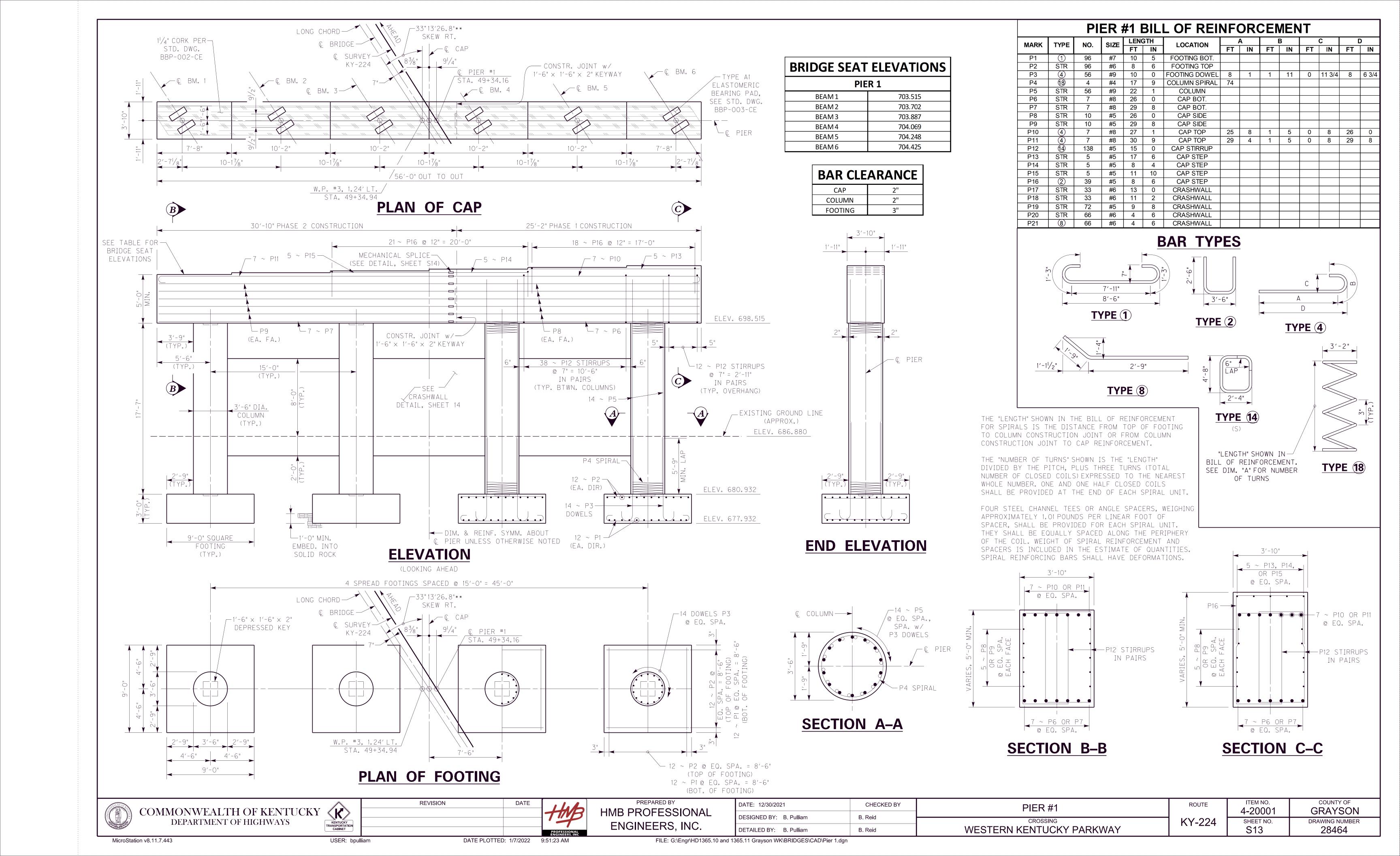
PREPARED BY HMB PROFESSIONAL ENGINEERS, INC.

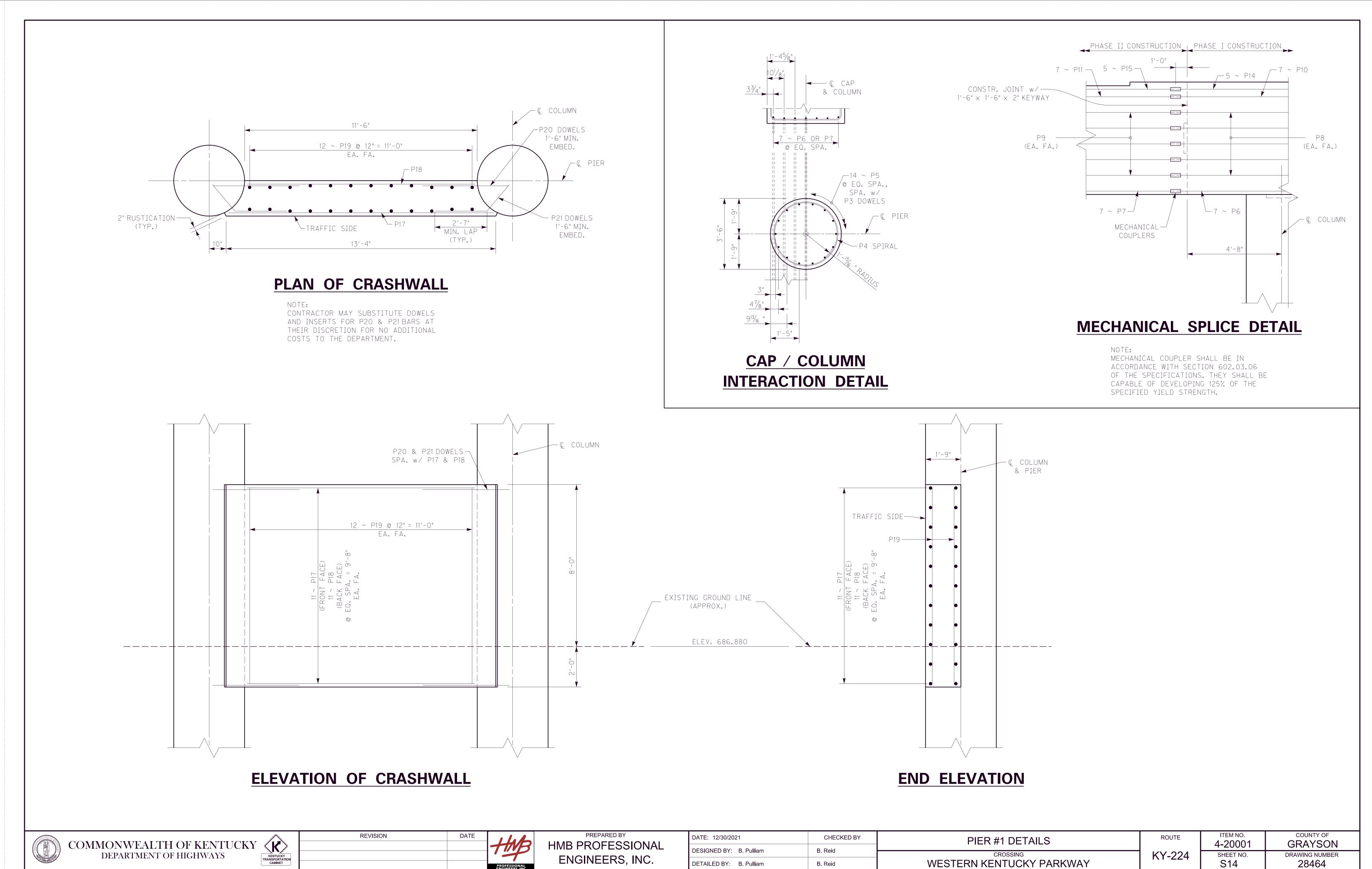
DESIGNED BY: B. Reid B. Pulliam DETAILED BY: B. Reid

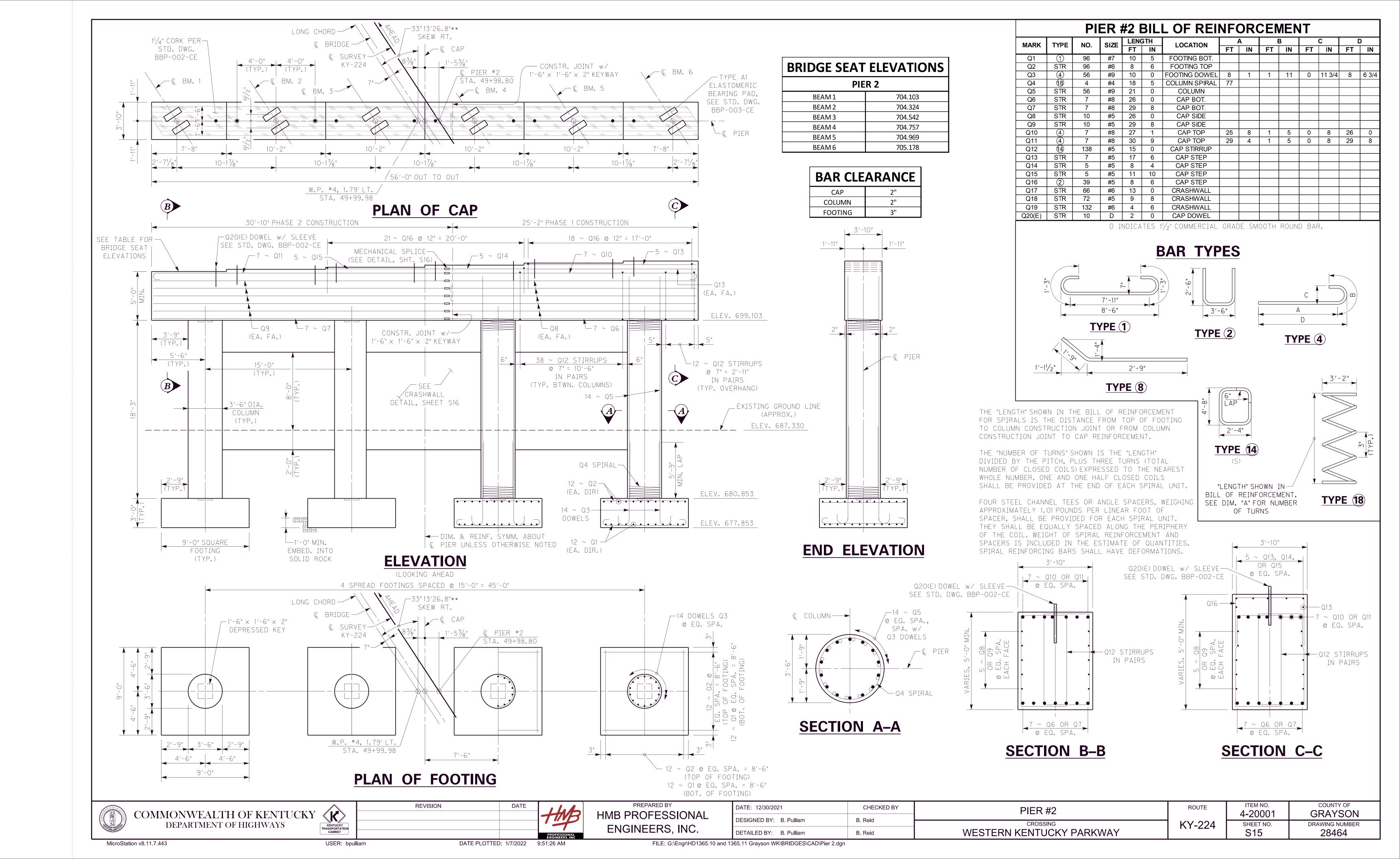
B. Pulliam

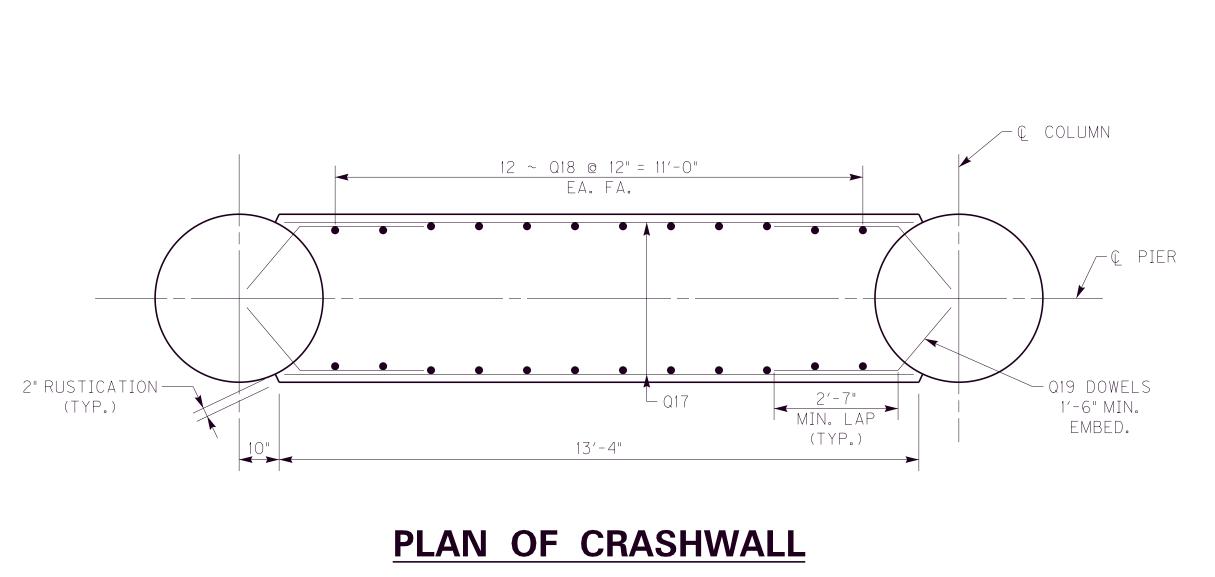
CROSSING WESTERN KENTUCKY PARKWAY KY-224

COUNTY OF 4-20001 GRAYSON DRAWING NUMBER SHEET NO. 28464

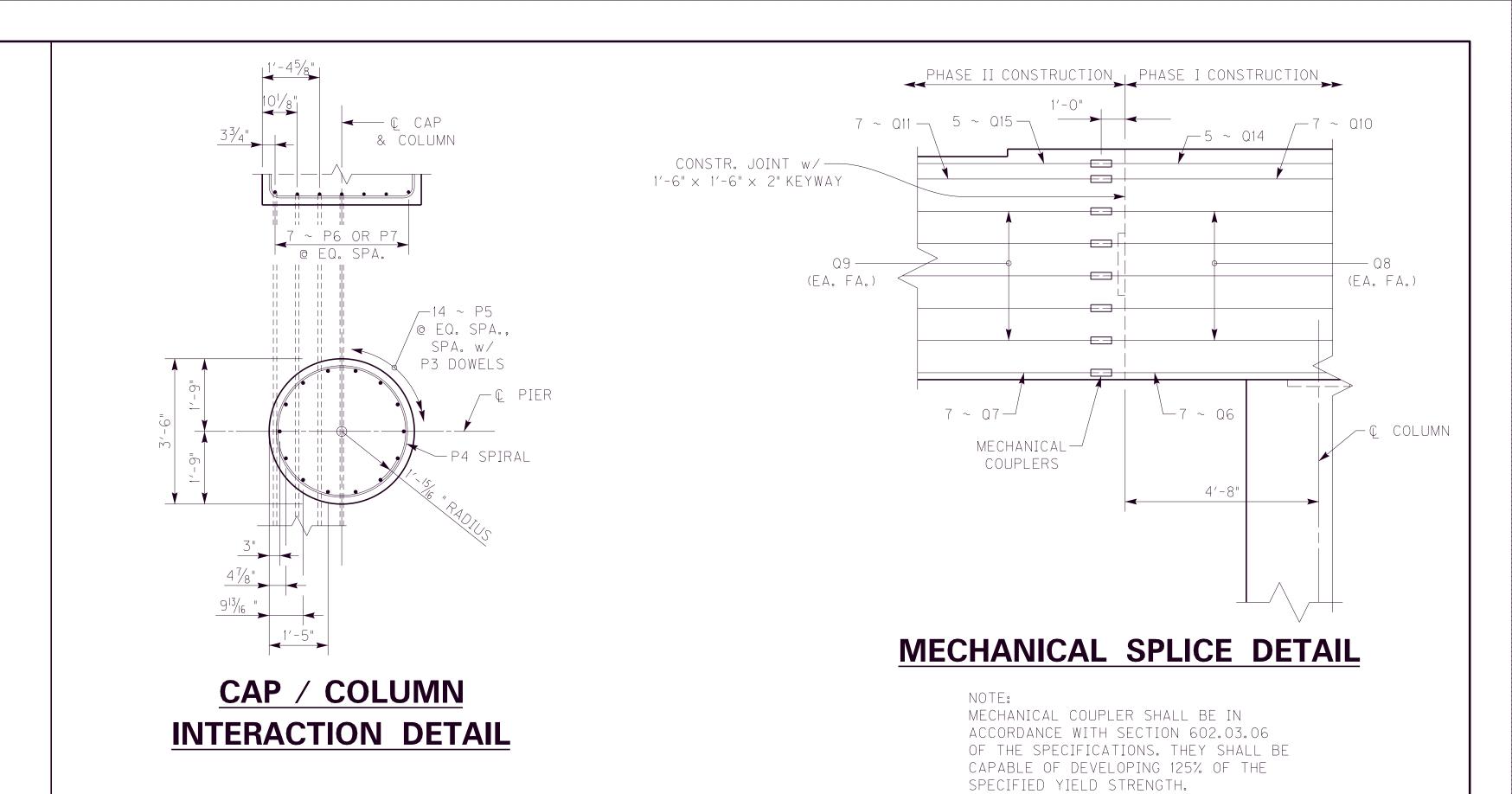


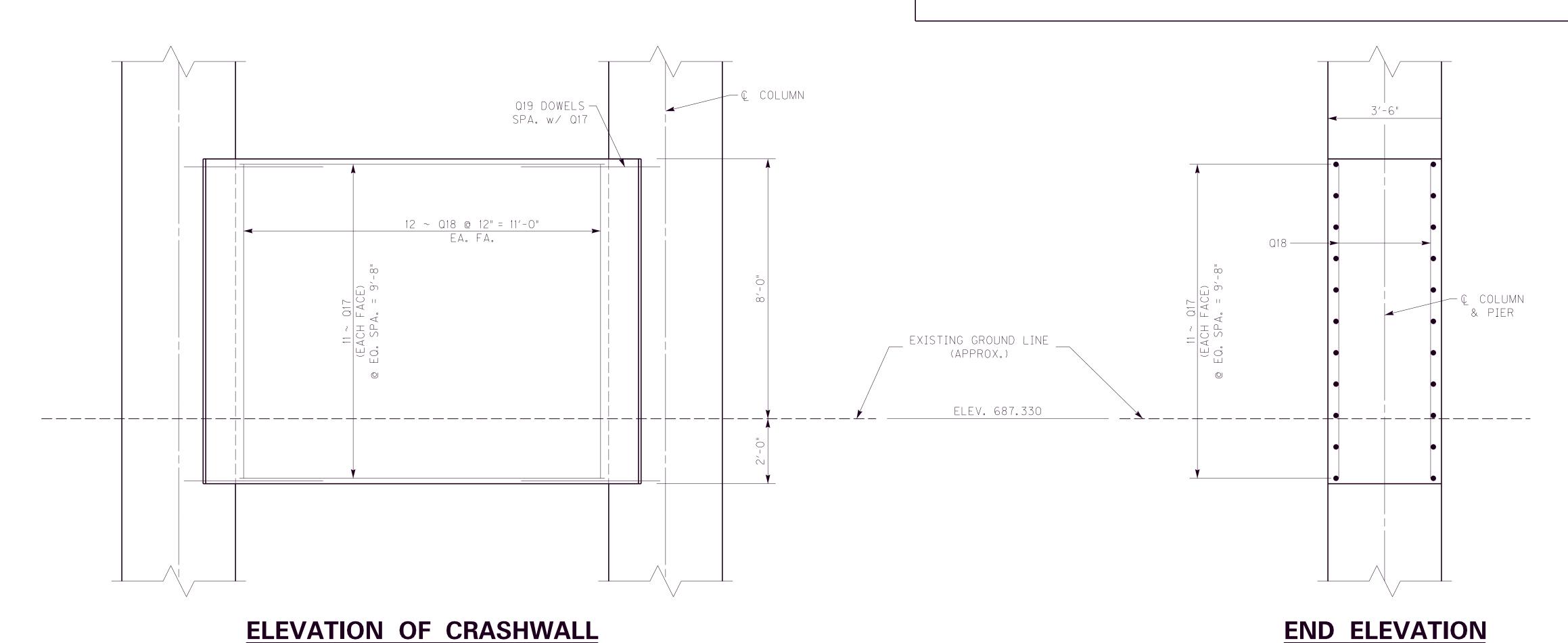






CONTRACTOR MAY SUBSTITUTE DOWELS AND INSERTS FOR Q19 BARS AT THEIR DISCRETION FOR NO ADDITIONAL COSTS TO THE DEPARTMENT.





COMMONWEALTH OF KENTUCKY
DEPARTMENT OF HIGHWAYS

KENTUCKY
TRANSPORTATION
CABINET

HMB PROFESSIONAL ENGINEERS, INC.

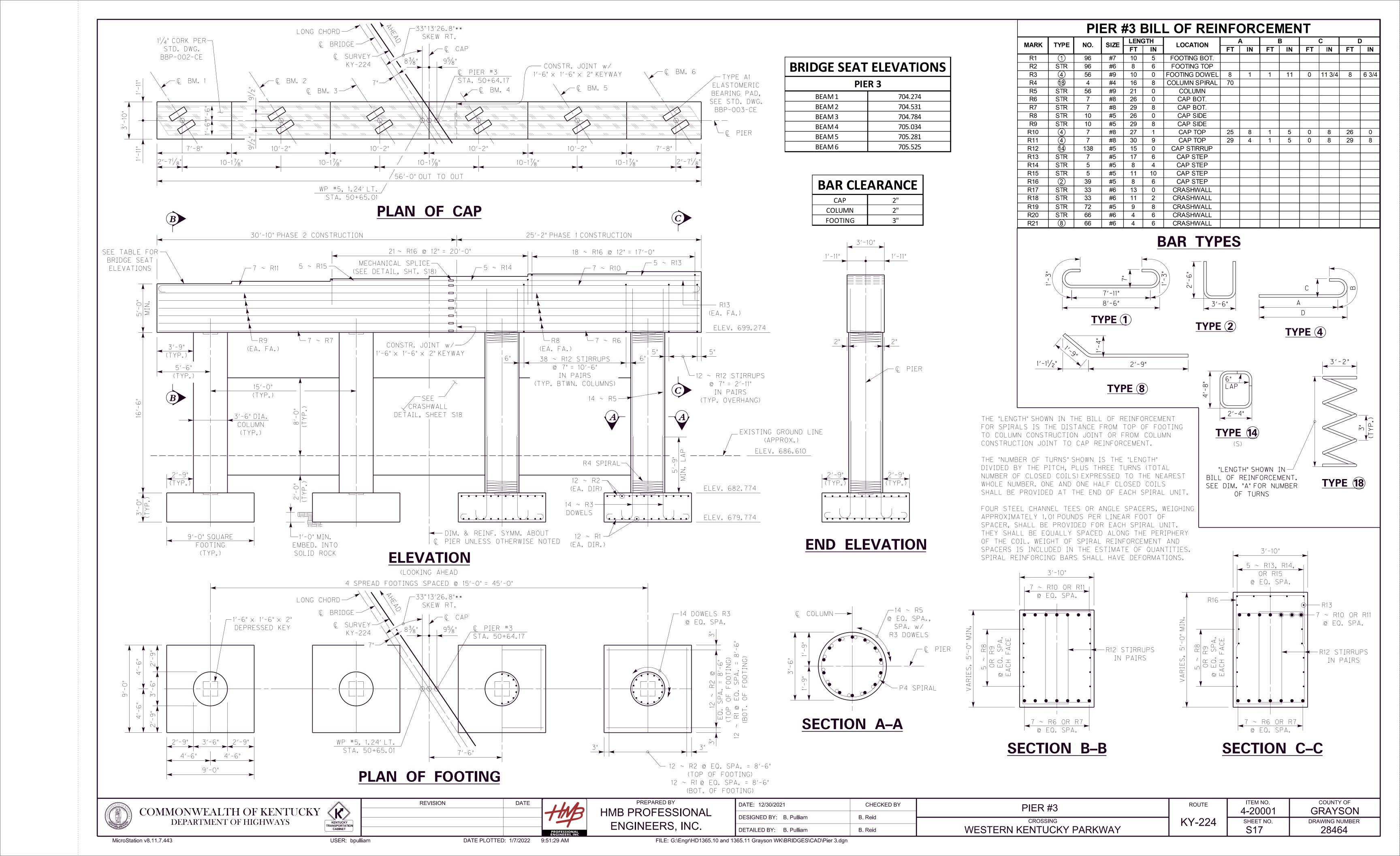
DATE: 12/30/2021 CHECKED BY PIER #2 DETAILS B. Reid WESTERN KENTUCKY PARKWAY DETAILED BY: B. Pulliam B Reid

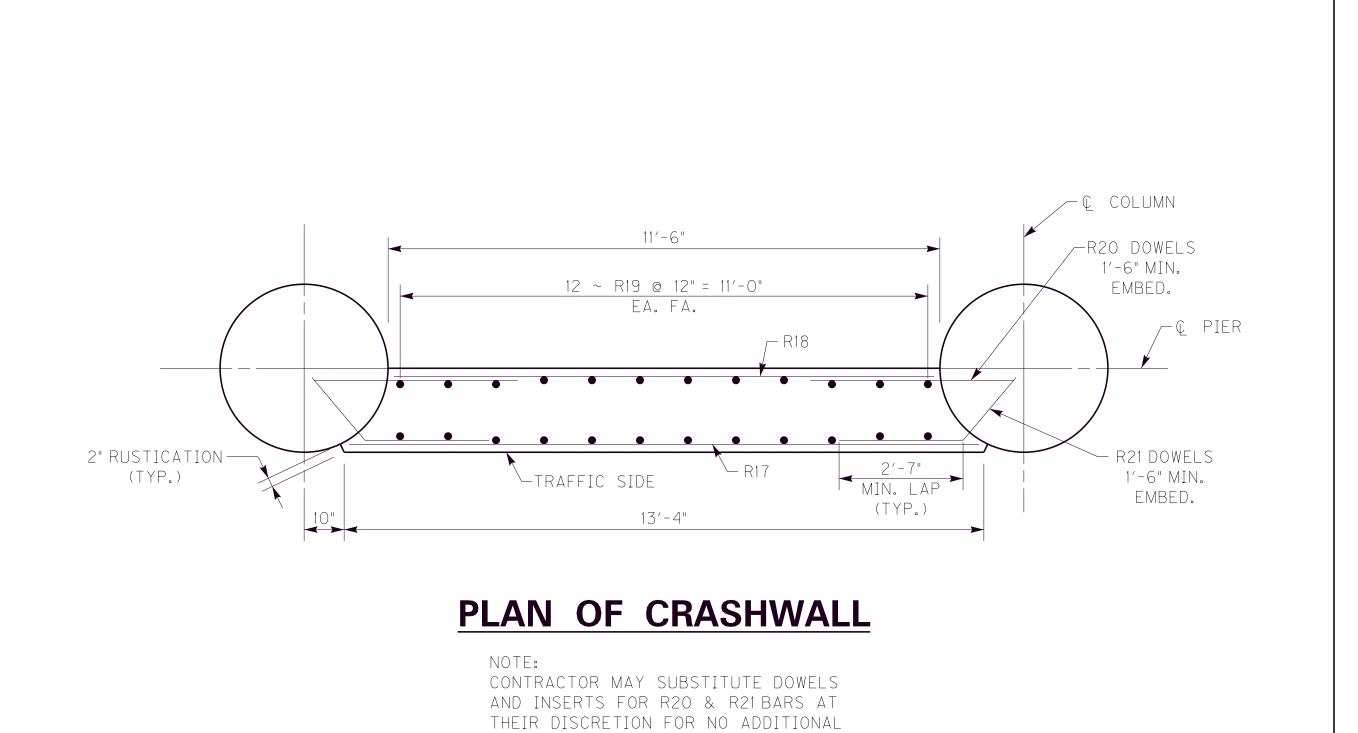
4-20001 KY-224 DRAWING NUMBER

COUNTY OF

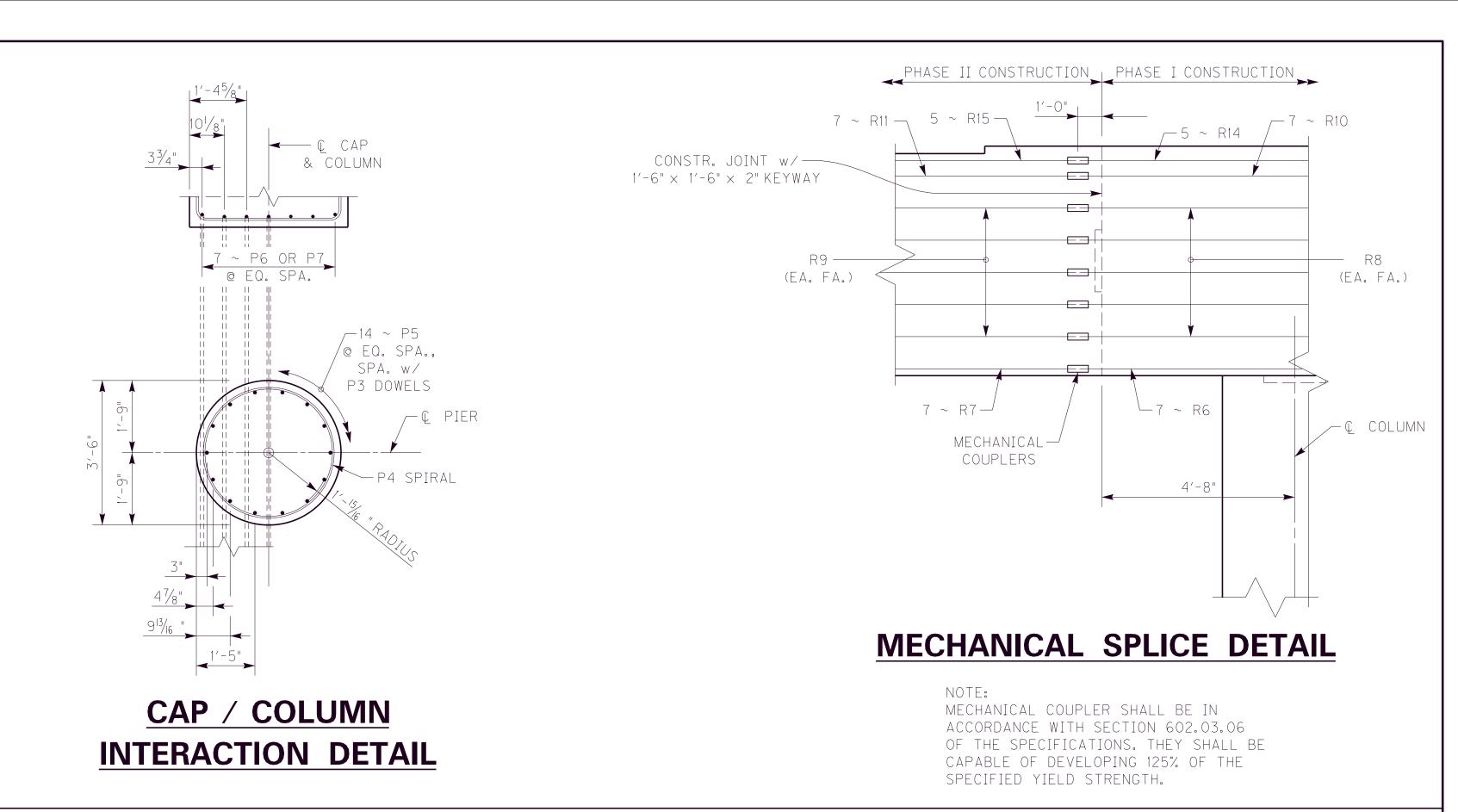
GRAYSON

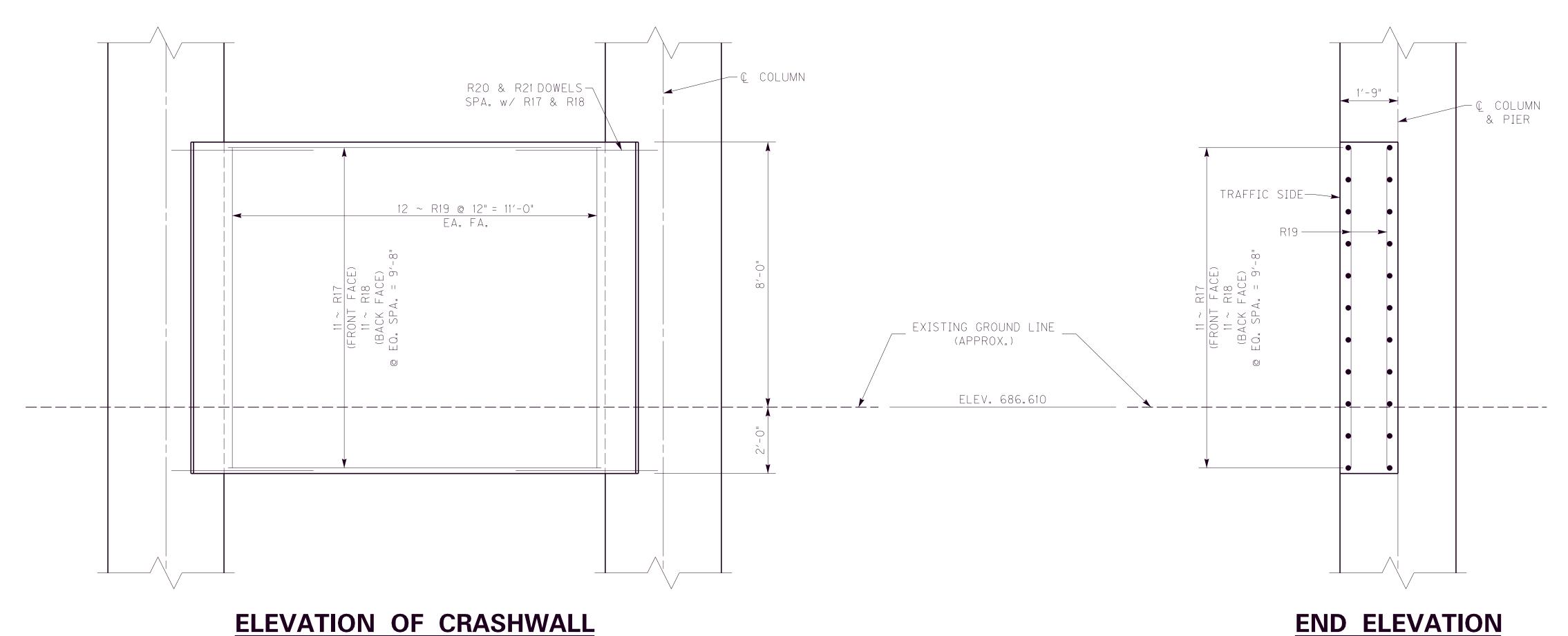
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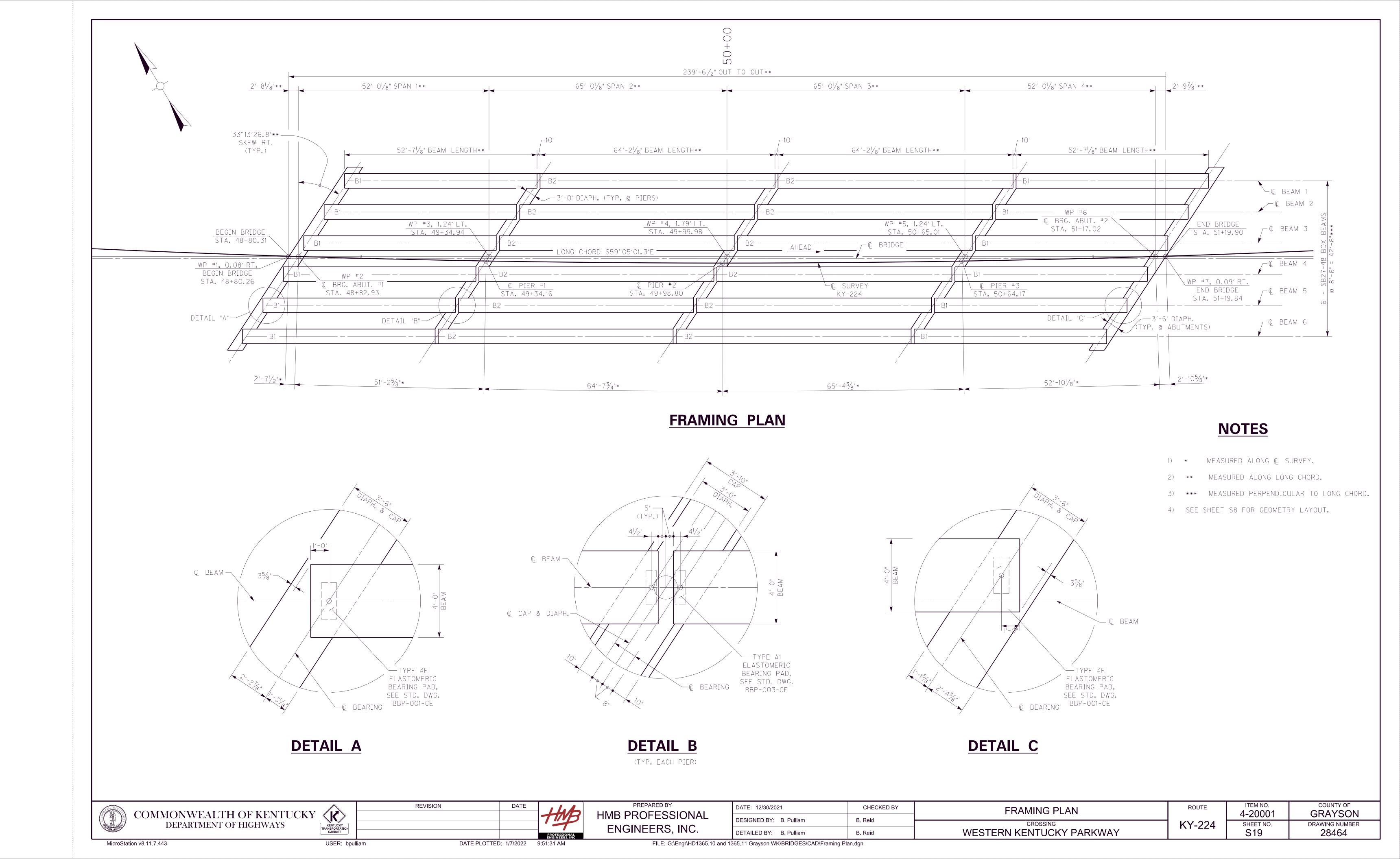


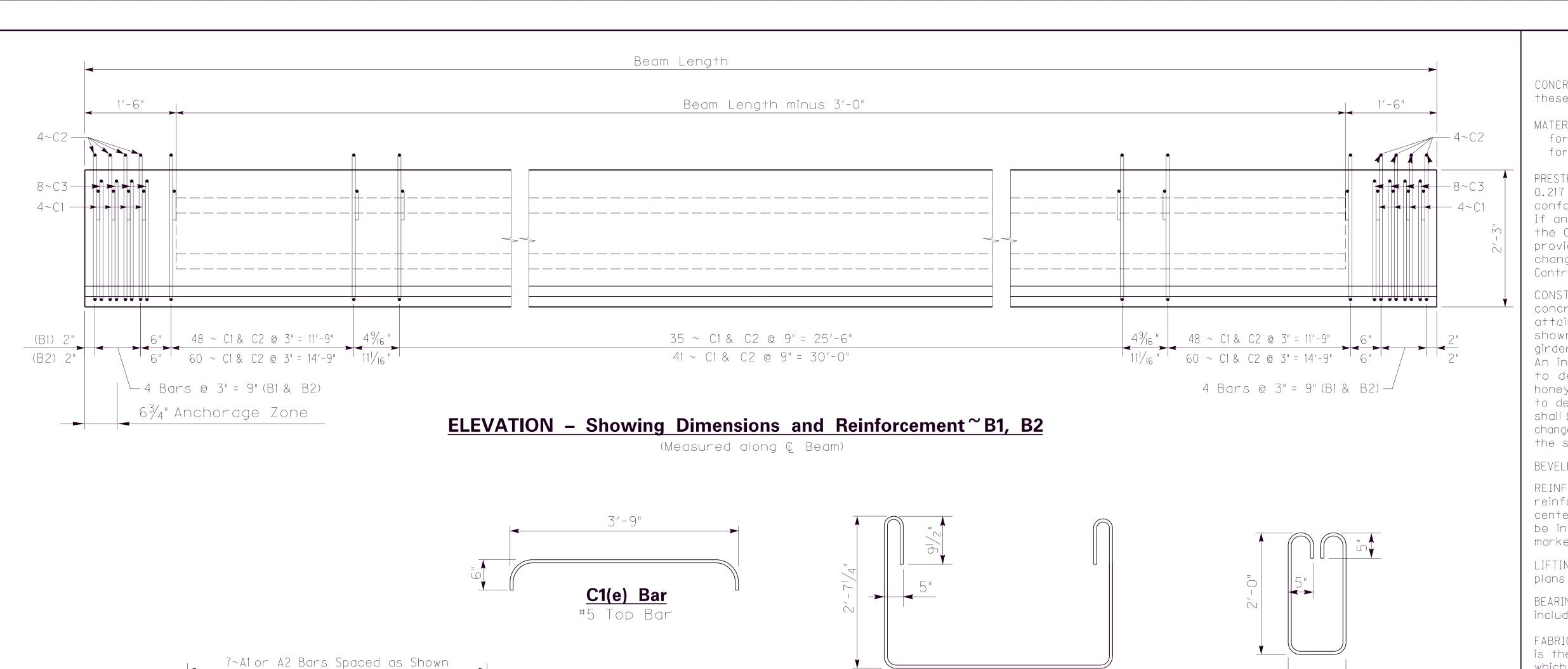


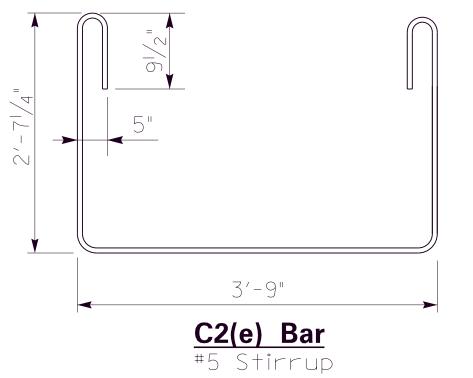
COSTS TO THE DEPARTMENT.

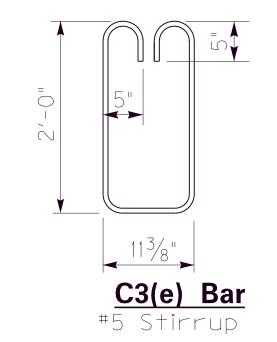












### **GENERAL NOTES**

CONCRETE: Ensure prestressed girder concrete is in accordance with these plans and the Specifications.

MATERIAL DESIGN SPECIFICATIONS:

FY = 60,000 psi for Steel Reinforcement for Prestressed Strand

F'S = 270,000 psi

PRESTRESSING STRANDS: Prestressing strands shall be 0.6" (nominal diameter, 0.217 sq. in.), uncoated seven-wire stress relieved, low-relaxation conforming to AASHTO M 203, Grade 270.

If an alternate strand arrangement or strand type is preferred by the Contractor, the designer that developed the original plans shall provide the design and also revise the original plans to reflect the changes. These design and plan modifications shall be done at the Contractor's expense.

CONSTRUCTION METHOD: No bond stress shall be transferred to the concrete, nor shall end anchors be released until the concrete has attained a minimum compressive strength of f'ci(shown in table) as shown by standard cylinders made and cured identically with the girders; f'c (shown in table) shall be attained at or prior to 28 days. An initial prestress force of 43,943 lbs. per low relaxation strand to develop a stress of 202,500 psi shall be applied. Beams with honeycomb of such extent as to affect the strength of resistance to deterioration will not be accepted. An allowance of .0005L (length) shall be made for shortening of beams due to shrinkage and elastic change. Shop plans shall show a detensioning plan by numbering in sequence, the strand pattern.

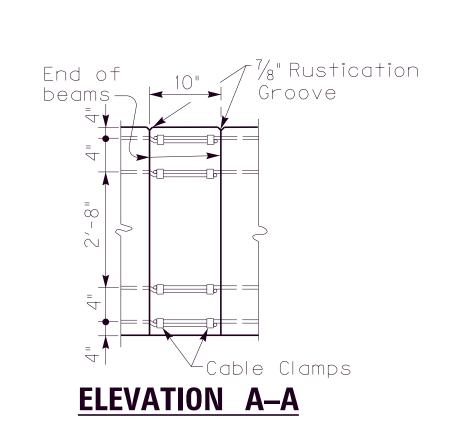
BEVELED EDGES: All exposed edges shall be beveled  $\frac{3}{4}$ ".

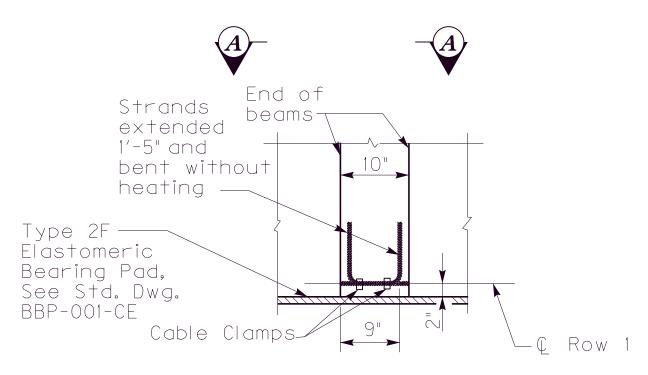
REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. Epoxy Coated Reinforcement shall be in accordance with Section 811.10 of the Specifications. All bars marked "C" shall be considered a stirrup for purpose of bend diameters.

LIFTING DEVICES: Dimensions for lifting devices shall be shown on shop plans for approval. Lifting shall be by equal loads to each device.

BEARING DEVICES: The cost of preformed cork and bearing pads shall be included in the price bid for per linear foot of beam.

FABRICATION: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab. If the measured camber is greater than the "Maximum Allowable Camber" the contractor will be responsible for any necessary adjustments to assure a minimum slab thickness of eight (8) inches as shown in the plans. This work will be considered incidental to the completion of the structure and have the approval of the Engineer.





STRAND SPLICING DETAIL

~Typical at Fixed Pier~

	Strand	d D	ata	with	n nur	nber i	ndic	ated	d in	row	S			В	ox Bea	m Data	a					troia	h+	Maximum
		Mi	dsp	an				End			Total	Concret	e Stress	_	Beam	Approx.		No.		Debond		traig force	ment	Allowable
Mark	F	Fully	Stres	ssed			Fully	Stre	ssed		T # of		si)	_# of	Length	Weight		C Ba	ars	Bosona	110111	.0.00		Camber
						(1) (2					Strands	f′ci	f′c	Beams		(lbs)	C1	C2	C3	D	Mark	Size	Length	
B1	13					13					13	5500	7000	12	52'-7 <sup>1</sup> / <sub>8</sub> "	41272	139	139	16	_	Δ1	#5	52′-4"	15/8"
B2	17					17					17	5500	7000	12	64'-21/8"	50562	169	169	16	_	Α2	#5	63′-11"	23/1"

1<sup>1</sup>/<sub>2</sub>"Cl.

© SB 27×48 — . ►

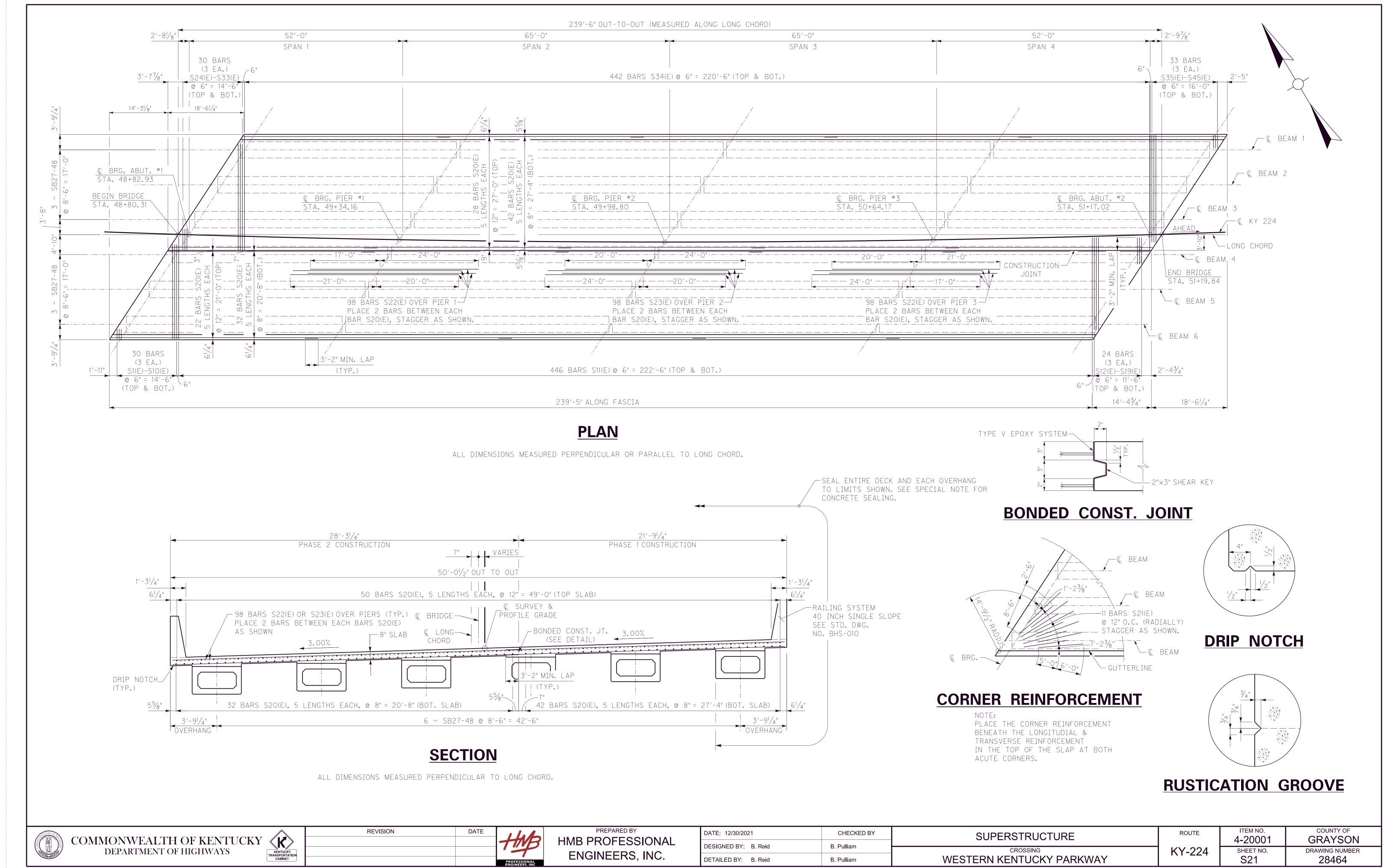
20 spa. @ 2"

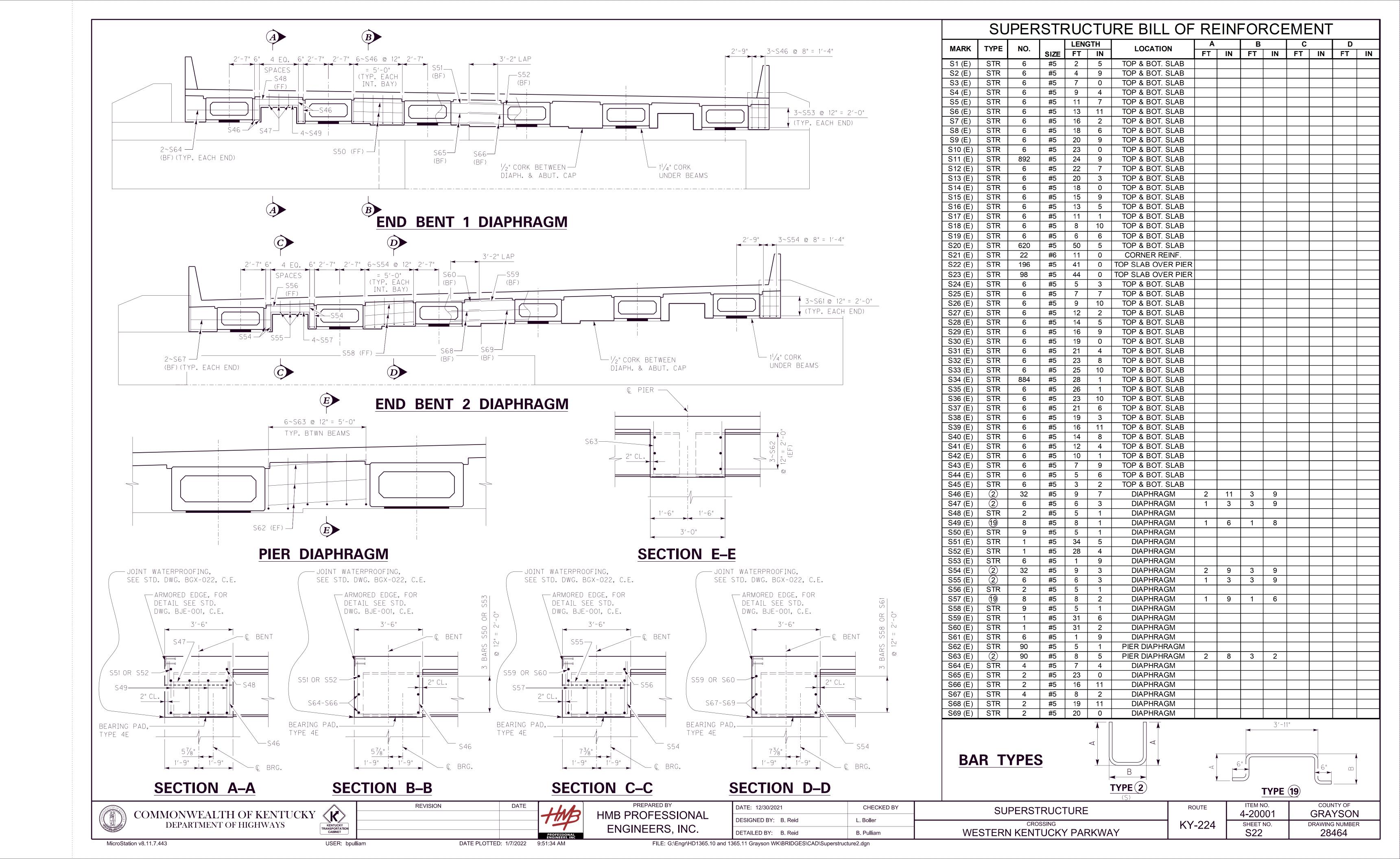
4'-0"

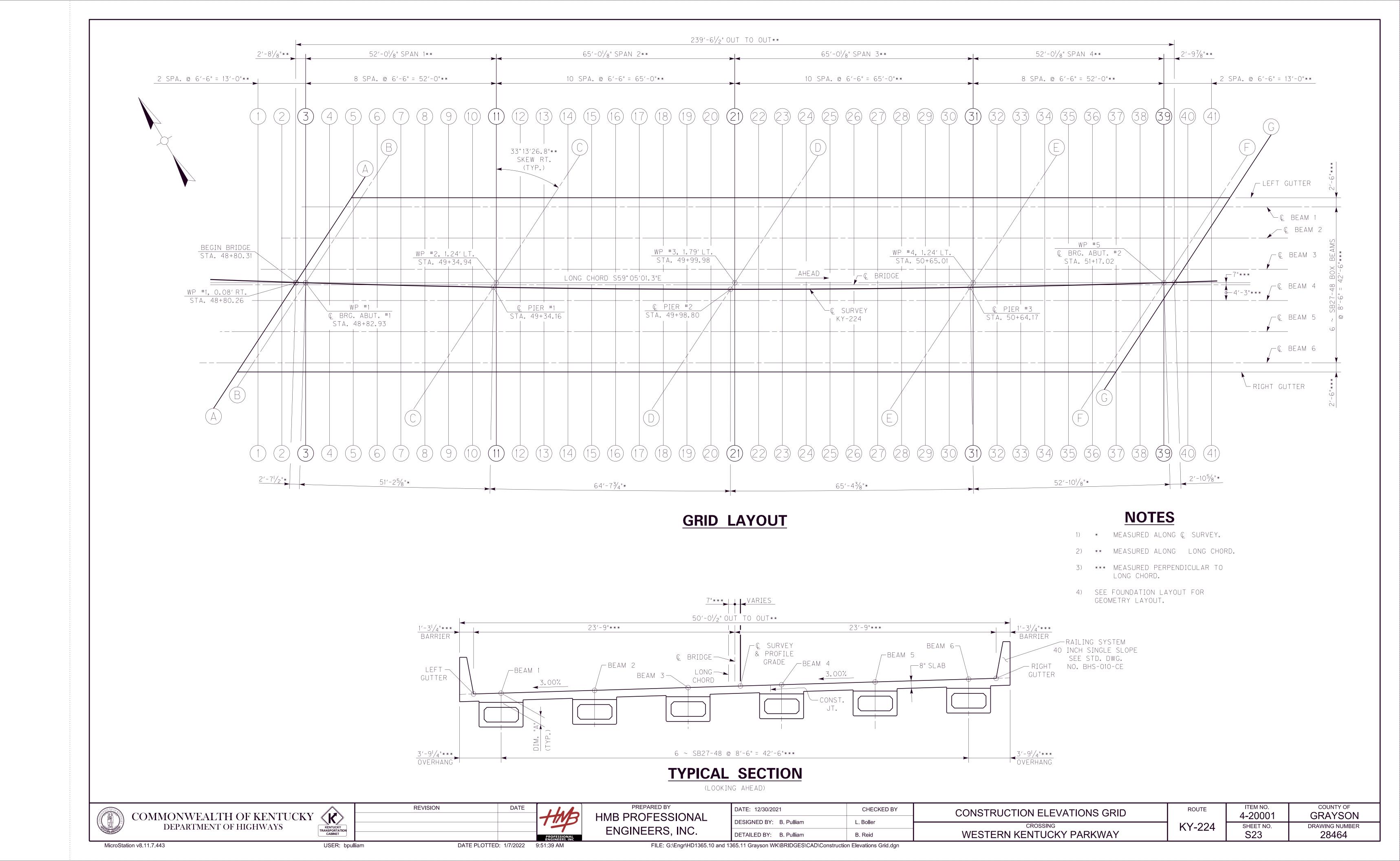
TYPICAL SECTION THROUGH BOX BEAM

Note: A2 Bars are to be lapped 2'-2" when necessary

COMMONWEALTH OF KENTUCKY (**)	REVISION DATE	46	PREPARED BY HMB PROFESSIONAL	DATE: 12/30/2021	CHECKED BY	SB27-48 BOX BEAM DETAILS	ROUTE	ITEM NO. <b>4-20001</b>	COUNTY OF GRAYSON
COMMONWEALTH OF KENTUCKY  DEPARTMENT OF HIGHWAYS  KENTUCKY  TRANSPORTATION		TIME		DESIGNED BY: B. Pulliam	L. Boller	CROSSING	KY-224	SHEET NO.	DRAWING NUMBER
TRANSPORTATION CABINET		PROFESSIONAL ENGINEERS, INC	ENGINEERS, INC.	DETAILED BY: B. Pulliam	L. Boller	WESTERN KENTUCKY PARKWAY		S20	28464







					K	Y-22	24	COI	NST	RI	JCTIC	ON E	LE	VAT	101	18							
LOCATION	LT.	BE	EAM 1		BE	AM 2		BE	EAM 3		PROFILE GRADE			BEAM 4			BEAM 5			BE	EAM 6		RT.
LOCATION	GUTTER	CONST. ELEV.	TOP OF BEAM		CONST. ELEV.	TOP OF BEAM		CONST. ELEV.	TOP OF BEAM	DIM "A"	CONST. ELEV.	TOP OF BEAM	DIM "A"	CONST. ELEV.	TOP OF BEAM	DIM "A"	CONST. ELEV.	TOP OF BEAM		CONST. ELEV.	TOP OF BEAM		GUTTER
AA	705.842	705.890	1		706.050			706.208			706.274			706.363			706.517			706.668			706.712
ВВ	705.889	705.937			706.098			706.257			706.325			706.414			706.568			706.721			706.765
CC	706.650	706.705			706.892			707.077			707.182			707.259			707.438			707.615			707.667
DD	707.227	707.293			707.514			707.732			707.870			707.947			708.159			708.368			708.430
EE	707.388	707.464			707.721			707.974			708.119			708.224			708.471			708.715			708.787
FF	707.217	707.302			707.589			707.872			707.993			708.151			708.428			708.700			708.780
GG	707.200	707.285			707.574			707.859			707.977			708.140			708.418			708.692			708.772
1			1							$\mathbf{I}$								<u> </u>		706.749			706.832
2																	706.617			706.890			706.971
3														706.481			706.754			707.025		1	707.103
4								706.341			706.464			706.614			706.883			707.150			707.227
5					706.199			706.470			706.597			706.740			707.004			707.265			707.340
6	705.973	706.053			706.324			706.592			706.721			706.855			707.114			707.369			707.443
7	706.095	706.174			706.440			706.703			706.834			706.961			707.214			707.464			707.537
8	706.208	706.286			706.548			706.804			706.937			707.057			707.306			707.550			707.622
9	706.311	706.388			706.644			706.895			707.030			707.143			707.388			707.641			707.725
10	706.405	706.480			706.730			706.977			707.114			707.221			707.481			707.765			707.848
11	706.488	706.562			706.808			707.051			707.197			707.318			707.601			707.882			707.962
12	706.562	706.634			706.878			707.150			707.317			707.433			707.713			707.986			708.066
13	706.629	706.700			706.980			707.263			707.429			707.540			707.814			708.080			708.156
14	706.723	706.806			707.088			707.364			707.528			707.636			707.901			708.160			708.234
15	706.827	706.908			707.184			707.455			707.617			707.719			707.975			708.225			708.297
16	706.921	707.001			707.270			707.532			707.690			707.788			708.036			708.278			708.348
17	707.003	707.081	_		707.343			707.596			707.751			707.843			708.083			708.318			708.386
18	707.072	707.148			707.401			707.646	<del> </del>		707.798			707.885			708.119	_		708.349			708.416
19	707.128	707.201			707.445			707.684			707.832			707.916			708.145	_		708.386			708.469
20	707.170	707.240			707.477			707.709			707.856			707.938			708.187	_		708.470			708.552
21	707.199	707.268			707.499		<u> </u>	707.727	<del></del>		707.884			707.986			708.267			708.546			708.626
22	707.216	707.283			707.511			707.779			707.961			708.060			708.338	_		708.610			708.689
23	707.226	707.293	_	1	707.569		<u> </u>	707.850			708.028			708.126			708.398			708.663			708.739
24	707.273	707.355			707.636			707.910			708.085			708.181			708.445			708.702			708.776
25	707.335	707.416		1	707.691		<u> </u>	707.960			708.129			708.223			708.478	_		708.726			708.798
26	707.387	707.467			707.735			707.996			708.159			708.251			708.498	_		708.738	<del>-</del>		708.808
27	707.427	707.505		_	707.766		<u> </u>	708.019			708.174			708.265			708.504			708.738			708.806
28	707.455	707.531		+	707.783			708.027		-	708.176			708.266			708.499			708.729			708.796
29	707.469	707.542		-	707.786		_	708.024	-	₩	708.165	-		708.256		-	708.484		_	708.721	-		708.800
30	707.470	707.540	4	-	707.776		<u> </u>	708.008	<u> </u>		708.145	<u> </u>		708.237			708.481	-	<u> </u>	708.749	<u> </u>		708.828
31	707.457	707.526		+	707.757		-	707.984		1	708.122	<u> </u>		708.235		-	708.503		-	708.769	-	_	708.846
32	707.433	707.500	_	-	707.728		<del> </del>	707.986		<del> </del>	708.138	<u> </u>		708.254		_	708.519	_		708.781	<u> </u>	-	708.857
33	707.400	707.467		+	707.732		-	708.000		╂	708.148	-		708.265		-	708.525	_	-	708.782	-	1	708.856
34	707.395	707.474		+	707.741		-	708.005			708.147			708.266		-	708.521			708.772		1	708.845
35	707.400	707.478		+	707.742		<u> </u>	708.001			708.136			708.257			708.507	_		708.753			708.826
36	707.396	707.474			707.733		1	707.987			708.115			708.237			708.484			708.727		1	708.798
37	707.384	707.460 707.435		1	707.714		-	707.963		+	708.082 708.041			708.209		+	708.452		1			-	
38 39	707.361			+	707.684	<b>.</b>	1	707.930 707.888	<del>-</del>	-	708.041			708.172								-	
	707.327	707.401		-	707.646		-	101.008		+						-		1	1			1	
40	707.285	707.356	<u> </u>		707.600	ļ		<u></u>											1			1	

## **NOTES**

TAKE ELEVATIONS ON TOP OF BEAM AT POINTS INDICATED BY THE GRID LAYOUT. THE BEAM ELEVATIONS ARE TO BE READ TO THREE DECIMALS AND ENTERED IN TABLES UNDER "TOP OF BEAM" ELEVATIONS. COMPUTE DIMENSION "A" AS FOLLOWS:

"CONSTRUCTION ELEVATION" MINUS "TOP OF BEAM ELEVATION" EQUALS DIMENSION "A". CONSTRUCTION ELEVATIONS INCLUDE CAMBER DUE TO WEIGHT OF CONCRETE SLAB AND BARRIER. MEASURING OF DIMENSION "A" GIVES THE FINAL CHECK ON BEAM TOLERANCES FOR CAMBER, BEAM DAMAGE AND ERRORS IN ERECTION THAT PRODUCE REVERSE CAMBERS, SAGS AND UNSIGHTLY FASCIA BEAMS.

FOR SETTING TEMPLATES, MEASURE DIMENSION "A" ABOVE TOP OF BEAMS FOR TOP OF TEMPLATE. DO NOT SET TEMPLATE BY ELEVATIONS.

TEMPORARY SUPPORTS OR SHORING WILL NOT BE PERMITTED UNDER THE BEAMS WHEN POURING THE CONCRETE FLOOR SLAB OR WHEN TAKING "TOP OF BEAM" ELEVATIONS.

CONSTRUCT BARRIER TO ROADWAY GRADE. DO NOT ADD CAMBER TO PARAPET.

NOTE TO RESIDENT: THE "MAXIMUM ALLOWABLE CAMBER" SHOWN ON THE BEAM SHEET IS THE AMOUNT OF CAMBER, MEASURED PRIOR TO CASTING THE DECK, ABOVE WHICH THE BEAM WILL BEGIN TO ENCROACH INTO THE SLAB. IF THE MEASURED CAMBER IS GREATER THAN THE "MAXIMUM ALLOWABLE CAMBER" THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY NECESSARY ADJUSTMENTS TO ASSURE A MINIMUM SLAB THICKNESS OF EIGHT (8) INCHES AS SHOWN IN THE PLANS. THIS WORK WILL BE CONSIDERED INCIDENTAL TO THE COMPLETION OF THE STRUCTURE AND HAVE THE APPROVAL OF THE ENGINEER.

REVISION



PREPARED BY HMB PROFESSIONAL ENGINEERS, INC.

DATE: 12/30/2021 CHECKED BY DESIGNED BY: B. Pulliam L. Boller DETAILED BY: B. Pulliam L. Boller

CONSTRUCTION ELEVATIONS TABLE WESTERN KENTUCKY PARKWAY

COUNTY OF 4-20001 GRAYSON KY-224 DRAWING NUMBER SHEET NO. 28464

ROUTE